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NATIONAL DAM INSPECTION PROGRAM, MINE NO. 60-POND 6 (INDI NUMBER--ETC(U)  
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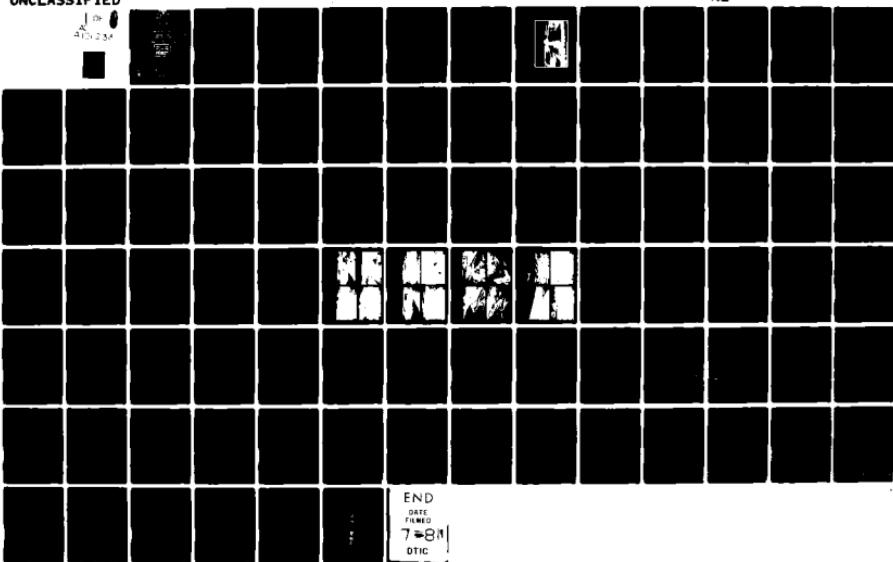
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LEVEL 1  
OHIO RIVER BASIN,  
TRIBUTARY TO CENTER BRANCH - PIGEON CREEK,  
WASHINGTON COUNTY

PENNSYLVANIA.

NDI No. PA 01145

PENN DER No. 63-89

⑥ National Dam Inspection Program.

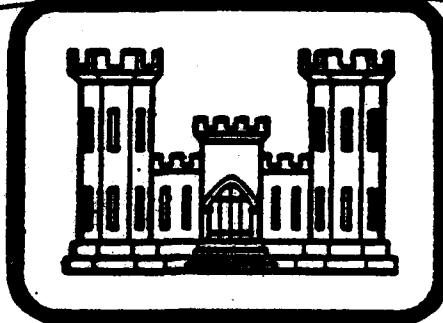
MINE NO. 60 - POND 6 (NDI number  
PA 01145, PENN DER Number 63-89)

REEDERS MINES CORPORATION  
REEDERS, PENNSYLVANIA

PHASE I INSPECTION REPORT.

NATIONAL DAM INSPECTION PROGRAM  
⑯ PACW 31-81-C-0027

JUL 10 1981



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PREPARED FOR

DEPARTMENT OF THE ARMY  
BALTIMORE DISTRICT, CORPS OF ENGINEERS  
BALTIMORE, MARYLAND 21203

BY

ACKENHEIL & ASSOCIATES GEO SYSTEMS, INC.  
CONSULTING ENGINEERS  
1000 BANKSVILLE ROAD  
PITTSBURGH, PENNSYLVANIA 15216

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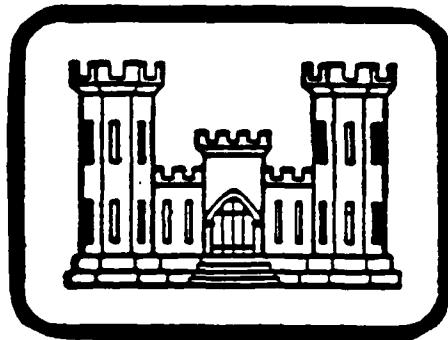
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OHIO RIVER BASIN

MINE NO. 60-POND 6  
WASHINGTON COUNTY, COMMONWEALTH OF PENNSYLVANIA  
NDI NO. PA 01145  
PennDER NO. 63-89

BETHLEHEM MINES CORPORATION  
ELLSWORTH-BUTLER DIVISION

PHASE I INSPECTION REPORT  
NATIONAL DAM INSPECTION PROGRAM



JUL 10 1981

Prepared for: DEPARTMENT OF THE ARMY  
Baltimore District, Corps of Engineers  
Baltimore, Maryland 21203

Prepared by: ACKENHEIL & ASSOCIATES GEO SYSTEMS, INC.  
Consulting Engineers  
1000 Banksburg Road  
Pittsburgh, Pennsylvania 15216

Date: May 1981

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## PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams for Phase I investigations. Copies of these guidelines may be obtained from the Department of the Army, Office of Chief of Engineers, Washington, D.C. 20314.

The purpose of a Phase I investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon visual observations and review of available data. Detailed investigations and analyses involving topographic mapping, subsurface investigations, materials testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify the need for such studies which should be performed by the owner.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of the dam depends on numerous and constantly changing internal and external factors which are evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some time in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I investigations are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the spillway design flood is based on the estimated "Probable Maximum Flood" (PMF) for the region (greatest reasonably possible storm runoff), or fractions thereof. The spillway design flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition, and the downstream damage potential.

PHASE I INSPECTION REPORT  
NATIONAL DAM INSPECTION PROGRAM

SYNOPSIS OF ASSESSMENT AND RECOMMENDATIONS

NAME OF DAM: Mine No. 60-Pond 6  
STATE LOCATION: Pennsylvania  
COUNTY LOCATION: Washington  
STREAM: Unnamed tributary to Center Branch  
of Pigeon Creek.  
DATE OF INSPECTIONS: 18 March and 26 March 1981  
COORDINATES: Lat. 40°08'56"  
Long. 80°03'30"

ASSESSMENT

Based on a review of available information and visual observations of conditions as they existed on the date of the field inspections, the general condition of the Mine No. 60-Pond 6 is considered to be good.

This assessment is based primarily on visual observations of the embankment and appurtenances and hydrologic/hydraulic analyses of reservoir/spillway capacity.

The structure is classified as an "intermediate" size, "high" hazard dam. Corps of Engineers guidelines recommend the Probable Maximum Flood (PMF) as the Spillway Design Flood for an "intermediate" size, "high" hazard dam. Mine No. 60-Pond 6's Spillway Design Flood is the Probable Maximum Flood. Spillway capacity is "adequate" because the non-overtopping flood discharge was found, by using the HEC-1 computer program, to be in excess of 100 percent of the PMF.

The Phase I investigation revealed deficiencies which should be corrected or improved through implementation of the following recommended remedial, monitoring and/or improvement efforts.

RECOMMENDATIONS

1. Outlet Works: Install the proposed outlet works as soon as possible, to provide a means for draining the impoundment zone.
2. Emergency Operation and Warning Plan: The owner should develop an Emergency Operation and Warning Plan including:

SYNOPSIS OF ASSESSMENT AND RECOMMENDATIONS (CONT'D)  
Mine No. 60-Pond 6

- a. Guidelines for evaluating inflow during periods of heavy precipitation or runoff.
- b. Procedures for around the clock surveillance during periods of heavy precipitation or runoff.
- c. Procedures for drawdown of the reservoir under emergency conditions.

d. Procedures for notifying downstream residents and public officials, in case evacuation of downstream areas is necessary.

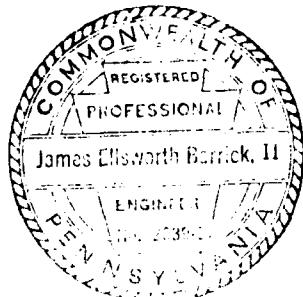
3. Remedial Work: The visual inspection of Mine No. 60-Pond 6 disclosed several minor deficiencies which should be corrected or monitored. These include:

- a. Improvements to downstream bench drainage to eliminate wet zones and embankment erosion conditions.
- b. Completing the embankment crest to design elevation. The crest should be graded to promote proper drainage and should be surfaced to prevent wheel rutting by vehicles that may traverse the crest.
- c. Implementation of a regularly scheduled monitoring program to observe the seepage at the downstream toe of the embankment for changes in water quality and/or quantity.

4. Follow-Up Remedial Work: Following the establishment of a vegetal cover on the embankment, spillway and adjacent slopes, backfill all erosional gullies and repair all surface slough zones and revegetate to achieve uniform, stable slopes.

SYNOPSIS OF ASSESSMENT AND RECOMMENDATIONS (CONT'D)  
Mine No. 60-Pond 6

Samuel G. Mazzella, P.E., 1981  
Samuel G. Mazzella  
Project Engineer



James P. Hannan, 8 May 1981  
James P. Hannan  
Project Engineer

James E. Barrick, 8 May 1981  
James E. Barrick, P.E.  
PA Registration No. 022639-E

Approved by:

James W. Peck  
James W. Peck  
Colonel, Corps of Engineers  
District Engineer

22 May 81  
Date

MINE No. 60 - POND 6



OVERVIEW

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PHASE I INSPECTION REPORT  
NATIONAL DAM INSPECTION PROGRAM  
MINE NO. 60-POND 6  
NATIONAL I. D. NO. PA 01145  
PennDER No. 63-89

SECTION 1  
PROJECT INFORMATION

1.1 GENERAL

a. Authority: The Phase I investigation was performed pursuant to authority granted by Public Law 92-367 (National Dam Inspection Act) to the Secretary of the Army, through the Corps of Engineers, to conduct inspections of dams throughout the United States.

b. Purpose: The purpose of the investigation is to make a determination on whether or not the dam constitutes a hazard to human life or property.

1.2 DESCRIPTION OF PROJECT

a. Dam and Appurtenances:

(1) Embankment: The Mine No. 60-Pond 6 was designed and constructed as a earthfill structure. The embankment is 845 feet long, with a toe to crest height of 68.2 feet and a crest width of about 20 feet. The embankment's upstream slope was observed to be 2H:1V above the waterline; the downstream slope was observed to be 2H:1V above the bench and 2.1H:1V below the bench.

(2) Principal (and Emergency) Spillway: The spillway for the Mine No. 60-Pond 6 consists of a 20 foot wide trapezoidal open channel in the left abutment.

(3) Outlet Works: An outlet works consisting of a submerged pump and pipeline is to be located near the rear of the impoundment. Water is to be returned to the Mine No. 60 Preparation Plant for reuse in the coal cleaning process. The outlet works was not installed at the time of the inspection.

(4) Freeboard Conditions: Freeboard between the the spillway crest and the reservoir pool level was 38.8 feet at the time of inspection.

(5) Downstream Conditions: The unnamed creek below the Mine No. 60-Pond 6 flows through an inhabited valley for about 1.5 miles to a confluence with the Center Branch of Pigeon Creek. Pigeon Creek flows into the Monongahela River near Monongahela, Pennsylvania. In the first 5 miles below the Mine No. 60-Pond 6, at least 5 inhabited dwellings, several major secondary roads, a railyard and a reservoir lie within the limits of the affected floodplain.

(6) Reservoir: The Mine No. 60-Pond 6 is about 1450 feet long at the spillway crest elevation and has a surface area of 21.1 acres. When the pool is at the (design) crest of the dam, the reservoir length increases to 1500 feet and the surface area is about 24.7 acres.

(7) Watershed: The watershed contributing to the Mine No. 60-Pond 6 is woodland and brushland. The watershed, 102 acres, is completely owned by the Bethlehem Mines Corporation.

b. Location: The Mine No. 60-Pond 6 is located at the headwaters of an unnamed tributary to the Center Branch of Pigeon Creek in Somerset Township, Washington County, Pennsylvania, approximately 3 miles north of Ellsworth, Pennsylvania.

c. Size Classification: The dam has a maximum storage capacity of 616 acre-feet and a toe to crest height of 68.2 feet. Based on the Corps of Engineers guidelines, this dam is classified as an "intermediate" size structure.

d. Hazard Classification: The Mine No. 60-Pond 6 is classified as a "high" hazard dam. In the event of a dam failure, at least 5 inhabited dwellings, several roads, a railyard and a reservoir could be subjected to damage, and the loss of more than a few lives could result.

e. Ownership: The Mine No. 60-Pond 6 is owned by the Bethlehem Mines Corporation. Correspondence can be addressed to:

Bethlehem Mines Corporation  
Ellsworth-Butler Division  
P. O. Box 143  
Eighty-Four, Pennsylvania 15330  
Attention: Mr. D. F. Patterson, Chief Engineer  
(412) 228-5500

f. Purpose of Dam: The Mine No. 60-Pond 6 was constructed to serve as a holding and settling impoundment for fine coal refuse slurry from the Mine No. 60 Coal Preparation Plant.

g. Design and Construction History: The dam was designed by Michael Baker, Jr., Inc. of Beaver, Pennsylvania, in 1980 and was constructed by Mack Equipment Corporation of Eighty-Four, Pennsylvania, from September through December 1980.

h. Normal Operating Procedures: The Mine No. 60-Pond 6 was designed to operate as an uncontrolled structure. Under normal operating conditions, the pool level (operating pool level) is maintained by the outlet works. The spillway defines the maximum operating pool elevation of about 1165.4. Inflow to the Mine No. 60-Pond 6 includes runoff from the watershed above and fine coal refuse slurry from the Mine No. 60 Preparation Plant.

1.3 PERTINENT DATA

a. <u>Drainage Area</u>	0.16 sq. mi.
b. <u>Discharge</u>	
Maximum flood at Dam Facility	Unknown
Spillway Capacity at Current Top of Dam	513 cfs
c. <u>Elevation (feet above MSL)</u>	
Design Top of Dam	1172.0*
Current Top of Dam (low point)	1169.5
Principal (and Emergency) Spillway	
Overflow Crest	1165.4
Operating Pool	Varies
Operating Pool on Date of Inspection	1126.6
Toe of Embankment	1101.3
d. <u>Reservoir Length</u>	
Length of Maximum Pool	1500 feet
Length of Maximum Operating Pool	1450 feet
Length of Operating Pool	Varies
e. <u>Reservoir Storage</u>	
Design Top of Dam	676 acre-feet
Current Top of Dam	616 acre-feet
Spillway Crest	525 acre-feet
f. <u>Reservoir Surface</u>	
Design Top of Dam	24.7 acres
Current Top of Dam	23.3 acres
Spillway Crest	21.1 acres
g. <u>Embankment</u>	
Type	Earth
Length	845 feet
Height	68.2 feet
Crest Width	20 feet
Slopes	
Downstream	2.0H:1V and 2.1 H:1V
Upstream	2.0 H:1V
Impervious Core	No*
Cutoff Provisions	Yes*
Grout Curtain	None reported

h. Principal (and Emergency) Spillway

Type	Trapezoidal Open Channel
Location	Left Abutment
Overflow Crest Length	20 feet
Crest Elevation	1165.4 feet

i. Outlet Works\*\*

Type	Submerged Pump*
Location	Right Side of Impoundment*

---

\* Taken or derived from available engineering drawings or reports.

\*\* Not installed at the time of the inspection.

SECTION 2  
ENGINEERING DATA

2.1 DESIGN

a. Data Available: The data available from the files of the Commonwealth of Pennsylvania, Department of Environmental Resources (PennDER), included a set of 17 construction drawings which are listed in Appendix B. Additional information was obtained from representatives of Bethlehem Mines Corporation.

b. Design History: Mine No. 60-Pond 6 was designed in 1980 by Michael Baker, Jr., Inc. of Beaver, Pennsylvania. A portion of the engineering was subcontracted to L. Robert Kimball and Associates of Ebensburg, Pennsylvania.

2.2 CONSTRUCTION

a. Construction: The dam was constructed between September and December 1980 by Mack Equipment Corporation of Eight-Four, Pennsylvania. Completion of the work is anticipated in 1981.

b. Modifications: There were no reported modifications to the dam design during construction.

2.3 OPERATION

The dam was designed to operate without a dam tender. The principal (and emergency) spillway is a 20 foot wide trapezoidal open channel in the left abutment.

The outlet works (to be installed) consists of an electric motor driven pump to return water to the Mine No. 60 Preparation Plant. At the time of the inspection there were no facilities to draw down the impoundment in the case of an emergency.

2.4 EVALUATION

a. Availability: Available design information and drawings were obtained from the Pennsylvania Department of Environmental Resources and were supplemented by conversations with company representatives.

b. Adequacy: The available engineering information was supplemented by field inspections and supporting engineering analyses and is considered adequate for the purpose of this Phase I Inspection Report.

c. Validity: Based on the review of the available information, there appears to be no reason to question the validity of the available engineering data.

## SECTION 3 VISUAL INSPECTION

### 3.1 FINDINGS

a. General: The field inspection of Mine No. 60-Pond 6 was performed on 18 March 1981 and 26 March 1981 and consisted of:

(1) Visual observations of the embankment crest and slopes, groins and abutments;

(2) Visual observations of the principal (and emergency) spillway including approach channel, overflow section, and discharge channel;

(3) Visual observations of the embankment's downstream toe area including drainage channels and surficial conditions;

(4) Visual observations of downstream conditions and evaluation of the downstream hazard potential;

(5) Visual observations of the reservoir shoreline and watershed;

(6) Transit stadia surveys of relative elevations along the embankment crest centerline, spillway, and across the embankment slopes.

The visual observations were made during a period when the reservoir pool was well below the normal operating level. The tailwater was at normal operating level during inspection.

The visual observations checklist, field sketch and sections containing the observations and comments of the field inspection team are contained in Appendix A. Specific observations are illustrated on photographs in Appendix C. Detailed findings of the field inspection are presented in the following sections.

#### b. Embankment:

(1) Crest: The crest of the embankment was straight throughout its length. No offsets or indications of misalignment were observed that would indicate anomalous movement of the embankment.

The crest was uneven vertically. The center and right portions of the crest appeared to be approximately at design elevation. The left end of the embankment was approximately 1 to 2 feet lower.

The embankment crest was barren earth and contained numerous vehicle ruts and considerable local unevenness.

(2) Upstream Slope: The upstream slope was generally even and uniform from top to bottom and abutment to abutment. Some minor surficial sloughing and erosion were noted as a result of a lack of vegetal cover. The sloughing appeared to be restricted to topsoil and seed bed mulch materials that covered the slope.

(3) Downstream Slope: The downstream slope was generally even and uniform from top to bottom and abutment to abutment. Like the upstream slope, the downstream slope contained a covering of topsoil and seed bed mulch, but had suffered considerably less erosion and sloughing distress.

The downstream slope contains a bench at approximately mid-height which is sloped to drain from the center toward each abutment. Ponded water was observed on the bench at the junctions with both abutments. Some erosion of the embankment slope has occurred below the ponded areas.

The groins (junction of embankment and abutment) contain a riprap erosion protection blanket that extends from the toe up both groins to within approximately 300 feet of the crest. No erosion of riprap or other distress was observed and the stone appeared to be durable material.

c. Drains: Two six inch diameter perforated CMP drain pipes were observed at the downstream toe of the embankment. No water was flowing from either pipe.

d. Abutments: The abutments below the embankment appeared to be in good condition. There were no signs of significant slope instability and only minor sloughing and minor erosion were noted. A small wet spot was observed on the slope along the toe drain channel approximately 200 feet below the dam, but no flowing water was observed.

e. Seepage: A flow of water estimated at 1 to 2 gallons per minute was observed in the toe drain channel. The origin of water appeared to be from below the groin riprap at the toe of the embankment. No sedimentation or movement of fine soil materials was observed.

f. Principal (and Emergency) Spillway:

(1) Approach Channel: The spillway approach channel was clear of obstructions that might reduce the capacity of the spillway.

(2) Overflow Crest: The spillway overflow crest is a level section in the channel at the centerline of the dam. The overflow crest was riprap covered and appeared to be in good condition on the date of inspection.

(3) Discharge Channel: The spillway discharge channel was riprapped for a distance of approximately 700 feet below the overflow crest. Below the riprap, the channel is an earth cut in original ground. The channel contained an erosional gully caused by surface runoff from above and beyond the dam. The depth of erosion ranged from 6 to 12 inches.

(4) Channel Side Slopes: The spillway channel side slopes contained some minor sloughing and considerable minor erosion as a result of surface runoff. The lack of vegetal cover appeared to be the main reason for the surface distress. There was no indication of more significant landslide activity that might reduce the capacity of the spillway channel.

f. Outlet Works: No outlet works was observed.

g. Reservoir:

(1) Slopes: The slopes of the reservoir ranged from mild to moderately steep and were generally barren. Some minor erosion of the unvegetated slopes has occurred. There were no indications of significant instability or erosion problems.

(2) Inlet Stream: Because of the location of the reservoir at the upper end of the watershed, there was no defined inlet stream to Pond 6.

(3) Sedimentation: None observed.

(4) Watershed: The watershed contributing to Pond 6 is entirely undeveloped and consists primarily of woods and brushland. There was no construction activity noted in the watershed on the date of inspection.

h. Downstream Conditions:

(1) Channel: The downstream channel for Pond 6 flows through an undeveloped valley for a distance of approximately 1.4 miles. In this reach, it passes beneath three township roads and a Conrail railyard at Mine No. 60. The channel joins the Center Branch of Pigeon Creek approximately 1.5 miles below the dam.

(2) Floodplain Conditions: In the first five miles below Pond 6 at least five inhabited dwellings lie on the floodplain at elevations low enough to possibly be affected by high flows.

### 3.2 EVALUATION

The following evaluations are based on the results of the visual inspections performed on 18 March and 26 March 1981.

a. Embankment: The condition of Mine No. 60-Pond 6 was good. Only minor deficiencies were observed which included:

(1) Water ponding and slope erosion near the ends of the bench in the downstream slope.

(2) An uneven profile and considerable wheel rutting and local unevenness of the crest.

(3) Minor erosion and sloughing of both the upstream and downstream slopes.

(4) Seepage at the downstream toe of the embankment.

b. Principal (and Emergency) Spillway: The spillway was in good condition. Only minor deficiencies were observed which included:

(1) An erosional gully in the lower discharge channel.

(2) Minor erosion and sloughing of spillway side slopes.

c. Outlet Works: No outlet works was observed. This is considered to be a deficiency.

d. Hazard Potential: Based on the observed downstream floodplain conditions, Mine No. 60-Pond 6 was assigned a "high" hazard potential rating.

SECTION 4  
OPERATIONAL FEATURES

**4.1 PROCEDURE**

Reservoir pool level is not currently maintained except by the overflow crest of the principal (and emergency) spillway.

The outlet works will reportedly consist of a submerged pump which will draw off pond water and return it to the Mine No. 60 Coal Preparation Plant. At the time of the inspection, the outlet works had not been installed.

Fine coal refuse slurry will be pumped to the facility for discharge to the pond across the right abutment near the embankment.

There are no pipelines through the embankment and there is no facility to drain the pond in case of an emergency.

Normal operating procedure does not require a dam tender.

**4.2 MAINTENANCE OF DAM**

The embankment and appurtenances are maintained by the Bethlehem Mines Corporation. Maintenance reportedly will consist of periodically repairing eroded areas and making miscellaneous repairs as necessary.

**4.3 INSPECTION OF DAM**

The Bethlehem Mines Corporation is required by the State of Pennsylvania to inspect the dam annually and make needed repairs.

The Bethlehem Mines Corporation is required by the Mine Safety and Health Administration (MSHA) to inspect the dam at least once every seven days and to make an annual report and certification of the dam.

**4.4 WARNING SYSTEM**

There is no warning system and no formal emergency procedure to alert or evacuate downstream residents upon threat of a dam failure.

**4.5 EVALUATION**

Lack of a facility to drain the reservoir in the case of an emergency is considered to be a deficiency.

There are no written operation, maintenance or inspection procedures, nor is there a warning system or formal emergency procedure for this dam. These procedures should be developed in the form of checklists and step by step instructions, and should be implemented as necessary.

SECTION 5  
HYDROLOGY AND HYDRAULICS

5.1 EVALUATION OF FEATURES

a. Design Data: Mine No. 60-Pond 6 has a watershed of 102 acres which is vegetated primarily by woodland and brushland. The watershed is about 2200 feet long and 1280 feet wide and has a maximum elevation of 1260 feet (MSL). At maximum normal pool, the dam impounds a reservoir with a surface area of 21.1 acres and a storage volume of 525 acre-feet. Maximum normal pool level is maintained at Elevation 1165.4 by the overflow crest of the principal (and emergency) spillway.

There were no calculations available concerning the spillway design capacity requirements at the time of design. Mine No. 60-Pond 6's actual spillway capacity, for the observed cross-section and existing freeboard conditions, was computed to be 513 cfs.

No additional hydrologic calculations were found relating reservoir/spillway performance to the Probable Maximum Flood or fractions thereof.

b. Experience Data: Records are not kept of reservoir level or rainfall amounts. There is no record or report of the embankment ever being overtopped.

c. Visual Observations: On the date of the field inspection, no serious deficiencies were observed that would prevent the principal (and emergency) spillway from functioning. The water level at the time of the field inspection was observed to be 38.8 feet below the spillway crest.

d. Overtopping Potential: Overtopping potential was investigated through the development of the Probable Maximum Flood (PMF) for the watershed and the subsequent routing of the PMF and fractions of the PMF through the reservoir and spillway. The Corps of Engineers guidelines recommends the Probable Maximum Flood for "intermediate" size, "high" hazard dams. Based on the observed size and existing downstream conditions, Mine No. 60-Pond 6 was assigned a Spillway Design Flood (SDF) of one PMF.

Hydrometeorological Report No. 33 indicates the adjusted 24 hour Probable Maximum Precipitation (PMP) for the subject site is 19.4 inches. No calculations exist that relate the reservoir/spillway system to a PMP type precipitation event. Consequently, an evaluation of the system was performed to determine whether or not the dam's spillway capacity is adequate under current Corps of Engineers' guidelines.

The Corps of Engineers, Baltimore District, has directed that the HEC-1 Dam Safety Version computer program be utilized. The program was prepared by the Hydrologic Engineering Center (HEC), U.S. Army Corps of Engineers, Davis, California, July 1978. The major methodologies and key input data for this program are discussed briefly in Appendix D.

The peak inflow to Mine No. 60-Pond 6 was determined by HEC-1 to be 650 cfs for the PMF (and SDF).

An initial pool elevation of 1165.4 was assumed prior to commencement of the storm.

e. Spillway Adequacy: The capacity of the combined reservoir and spillway system was determined to be in excess of the PMF by HEC-1. According to Corps of Engineers' guidelines, Mine No. 60-Pond 6's spillway is "adequate."

SECTION 6  
STRUCTURAL STABILITY

6.1 AVAILABLE INFORMATION

a. Design and Construction Data: All available design documentation, calculations and other data received from the Pennsylvania Department of Environmental Resources were reviewed. This data is discussed in Section 2 and a detailed listing is included in Appendix B. PennDER provided the drawings that are presented in Appendix E.

b. Operating Records: There are no written operating records or procedures for this dam.

c. Visual Observations:

(1) Embankment: Visual observations made during the field inspection did not indicate evidence of a high ground water level in the embankment. There was no pronounced "line of seepage" and no significant surface sloughs were observed.

The upstream slope was generally even and uniform. Some minor surficial sloughing and erosion were noted, apparently as a result of a lack of vegetal over. The sloughing appeared to be restricted to topsoil and seed bed mulch materials that covered the slope.

The downstream slope was generally even and uniform and showed less erosion and sloughing than the upstream slope. Ponded water was observed standing in the bench on the downstream slope.

Some seepage was noted in the toe drain channel at the immediate downstream toe of the embankment. Origin of the water could not be determined. Embankment and foundation drains located immediately behind and 1.5 feet above the seepage area were dry on the dates of the inspection.

The stadia survey revealed a moderate downstream slope at about 2.2H:1V including downstream bench.

(2) Outlet Facilities: The outlet facilities were not installed at the time of the inspection.

(3) Principal (and Emergency) Spillway: The spillway discharge channel contained an erosional gully caused by surface runoff from above and beyond the dam. The channel side slopes contained some minor sloughing and considerable minor erosional distress as a result of surface runoff.

## 6.2 EVALUATION

a. Design, Construction, and Monitoring Information: The design documentation was, by itself, considered inadequate to evaluate the structure.

Stability analyses by Michael Baker, Jr., Inc., calculated safety factors against sliding for several embankment conditions. The results of these analyses are presented on Plate X in Appendix E.

b. Embankment: On the date of the field inspection, the embankment appeared to have an adequate margin of safety against sliding. This is based on observations of embankment slopes, materials, and seepage conditions.

c. Principal (and Emergency) Spillway: The principal (and emergency) spillway appeared to be functional. There was no indication of significant landslide activity that might imperil the capacity of the spillway channel.

d. Active Outlet Works: The outlet works was not installed at the time of inspection.

f. Seismic Stability: According to the Seismic Risk Map of the United States, Mine No. 60-Pond 6 dam is located in Zone 1 where damage due to earthquakes would most likely be minor.

A dam located in Seismic Zone 1 may be assumed to present no hazard from an earthquake provided static stability conditions are satisfactory and conventional safety margins exist. No calculations were developed to verify this assessment, however.

SECTION 7  
ASSESSMENT AND RECOMMENDATIONS

7.1 ASSESSMENT

a. Evaluation:

(1) Embankment: Mine No. 60-Pond 6's embankment is considered to be in good condition. This is based on visual observations that revealed only minor deficiencies.

(2) Principal (and Emergency) Spillway: The spillway is considered to be in good condition. This is based on visual observations that revealed only minor deficiencies and a HEC-1 analysis that indicated an "adequate" capacity rating under current Corps of Engineers guidelines.

The Spillway Design Flood for Mine 60-Pond 6 was the Probable Maximum Flood. The combined reservoir/spillway capacity was determined by HEC-1 to be in excess of 100% of the PMF.

(3) Outlet Works: Lack of an outlet works or facility for draining the pond is considered to be a deficiency.

(4) Seepage: Seepage at the downstream toe of the embankment represents a potential threat to the impoundment. However, on the date of inspection, there was no indication of excessive flow or movement of embankment or foundation soil materials.

(5) Emergency Plans: The lack of a documented emergency operation and warning plan is considered to be a deficiency.

b. Adequacy of Information: The information available on design, construction, operation and performance history in combination with visual observations and hydrology and hydraulic calculations was sufficient to evaluate the embankment and appurtenant structures in accordance with the Phase I investigation guidelines.

c. Urgency: The recommendations presented in Section 7.2a, 7.2b and 7.2c should be implemented immediately.

d. Necessity for Additional Studies: None.

7.2 RECOMMENDATIONS

a. Install Outlet Works: Install the proposed outlet works as soon as possible, to provide a means for draining the impoundment zone.

b. Emergency Operation and Warning Plan: The owner should develop an Emergency Operation and Warning Plan including:

(1) Guidelines for evaluating inflow during periods of heavy precipitation or runoff.

(2) Procedures for around the clock surveillance during periods of heavy precipitation or runoff.

(3) Procedures for drawdown of the reservoir under emergency conditions.

(4) Procedures for notifying downstream residents and public officials, in case evacuation of downstream areas is necessary.

c. Remedial Work: The visual inspection disclosed several minor deficiencies which should be corrected or monitored. These include:

(1) Improvements to downstream bench drainage to eliminate wet zones and embankment erosion conditions.

(2) Completing the embankment crest to design elevation. The crest should be graded to promote proper drainage and should be surfaced to prevent wheel rutting by vehicles that may traverse the crest.

(3) Implementation of a regularly scheduled monitoring program to observe the seepage at the downstream toe of the embankment for changes in water quality and/or quantity.

d. Follow-Up Remedial Work: Following the establishment of a vegetal cover on the embankment, spillway and adjacent slopes, backfill all erosional gullies and repair all surface slough zones and revegetate to achieve uniform, stable slopes.

APPENDIX A  
VISUAL INSPECTION CHECKLIST

VISUAL OBSERVATIONS CHECKLIST I  
(NON-MASONRY IMPOUNDING STRUCTURE)

Name of Dam	<u>Mine No. 60-Pond 6</u>	County	<u>Washington</u>	State	<u>Pennsylvania</u>	National ID #	<u>PA 01145</u>
Type of Dam	<u>Earth</u>						
Dates of Inspection	<u>18 March 1981</u>						
	<u>26 March 1981</u>	Hazard Category	<u>High</u>				
		Partly cloudy, cool	<u>35°F</u>	Temperature	<u>35°F</u>		
		<u>Clear, mild</u>	<u>45°F</u>				
Pool Elevation at Time of Inspection	<u><math>\pm 1126.6</math></u>	(MSL)					
Tailwater at Time of Inspection	<u><math>\pm 1101.3</math></u>	(MSL)					

Inspection Personnel: 18 March 1981  
 J. P. Hannan  
 S. G. Mazzella

Ackenheil & Associates, Geotechnical Engineer  
 Ackenheil & Associates, Civil Engineer

26 March 1981  
 J. E. Barrick, P.E.  
 and Hydrologist  
 J. L. Loveland  
 Bethlehem Mines Corporation Representative

Recorder J. E. Barrick

GEO Project G80138-G  
 PennDER I.D. No. 63-89

EMBANKMENT

<u>VISUAL EXAMINATION OF</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
<u>SURFACE CRACKS</u>	Randomly oriented drying cracks observed on downstream bench. No other cracks observed.	
<u>UNUSUAL MOVEMENT OR CRACKING AT OR BEYOND THE TOE</u>	None observed.	
<u>SLoughing OR Erosion OF EMBANKMENT AND ABUTMENT SLOPES</u>	No significant sloughing or erosion of embankment slopes was observed. Some minor sloughing of seed bed mulch and topsoil was noted. Vegetal cover has not yet been established. Some minor erosion also observed.	
	No significant sloughing or erosion of abutment slopes was observed. As with the embankment, some minor topsoil and mulch sloughing and erosion were noted due to a lack of an established vegetal cover.	
<u>VERTICAL AND HORIZONTAL ALIGNMENT OF THE CREST</u>	The horizontal alignment of the crest appeared to be straight.	The vertical alignment of the crest was uneven. The center and right portions of the crest were reasonably level and appeared to be at the required elevation. The left portion of the crest was 1 to 2 feet lower than the center and right. The company representative indicated that completion of the crest to design elevation would take place as soon as the current labor uncertainty was resolved.

EMBANKMENT (CONTINUED)

<u>VISUAL EXAMINATION OF</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
RIPRAP FAILURES	None observed.	
SETTLEMENT	None observed.	
JUNCTION OF EMBANKMENT AND ABUTMENT	The left upstream groin (junction of embankment and abutment) contains minor erosion due to channeling of surface runoff. The remaining three groins appeared to be in good condition with no indications of erosion or instability.	
JUNCTION OF EMBANKMENT AND SPILLWAY	No junction observed.	
ANY NOTICEABLE SEEPAGE	The toe drain channel below the embankment contained flowing water estimated at approximately 1 to 2 gallons per minute. Most, if not all, of the flowing water appeared to originate at the riprap groin protection at the downstream toe of the dam. There was no indication of discoloration or deposition of fine soil materials at or below the origin of the seepage.	
	A wet spot was noted on the toe drain channel left slope approximately 30 feet below the toe of the dam. The cause of the wet spot could not be determined.	
STAFF GAUGE AND RECORDER	None observed.	

EMBANKMENT (CONTINUED)

<u>VISUAL EXAMINATION OF</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
<b>DRAINS</b>	Two six inch diameter perforated CMP drain pipes exit the downstream slope and discharge to the riprap groin protection at the toe of the dam. No flows were observed on the date of inspection.	
<b>SURFICIAL CONDITIONS</b>	The embankment crest was barren earth on the date of inspection. It contained numerous vehicle ruts and was locally irregular and uneven.	<p>The upstream slope was generally uniform from top to bottom and abutment to abutment and was uniformly covered with a straw mulch placed to assist development of a vegetal cover. Numerous small mulch and topsoil sloughs were observed and numerous minor shallow erosion channels were noted. These appeared to be the result of a lack of vegetal cover.</p> <p>The downstream slope appeared to be generally uniform and regular from top to bottom and abutment to abutment. As with the upstream slope, the downstream slope was covered with a straw mulch. The downstream slope appeared to have suffered considerably less sloughing and erosion than the upstream slope.</p> <p>The bench on the downstream slope was sloped to drain from the center to each side of the embankment. Ponded water was noted at each end of the bench where the bench intersected the riprap groin protection. In both cases, an erosion drainage channel has been cut from the ponded area over the crest of the bench onto the lower downstream slope.</p>

PRINCIPAL (AND EMERGENCY) SPILLWAY

<u>VISUAL EXAMINATION OF</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
APPROACH CHANNEL	The spillway approach channel was clear of major obstructions that would reduce its capacity during periods of high flow.	
OVERFLOW SECTION	The spillway overflow section occurs at the intersection of the dam centerline and consists of a riprap lined level stretch in the spillway channel. The overflow section was clear of obstructions and appeared to be capable of maintaining design flows.	
DISCHARGE CHANNEL	The spillway discharge channel was unobstructed through its entire length between the overflow section and its confluence with the toe drain channel below the dam. Like the overflow section, the discharge channel was riprap lined for approximately 700 feet beyond the dam centerline.	
SIDE SLOPES	The spillway side slopes from the approach channel through discharge channel have suffered some minor erosion and minor topsoil and mulch sloughing.	

INSTRUMENTATION

<u>VISUAL EXAMINATION OF</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
MONUMENTATION/SURVEYS	None observed.	
WEIRS	None observed.	
PIEZOMETERS	None observed.	
OBSERVATION WELLS	None observed.	

RESERVOIR

<u>VISUAL EXAMINATION OF</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
<b>SLOPES</b>	The slopes of the reservoir ranged from gentle to moderately steep and generally consisted of barren earth. Some minor erosion was noted in all areas without vegetation. There were no indications of reservoir slope instability.	
<b>SEDIMENTATION</b>	None observed.	
<b>INLET STREAM</b>	Pond 6 is located near the top of the watershed and has no defined inlet stream.	
<b>WATERSHED</b>	The watershed above Pond 6 is undeveloped and generally tree and brush covered. On the date of inspection there were no signs of construction activity in the contributing watershed.	

DOWNSTREAM CONDITIONS

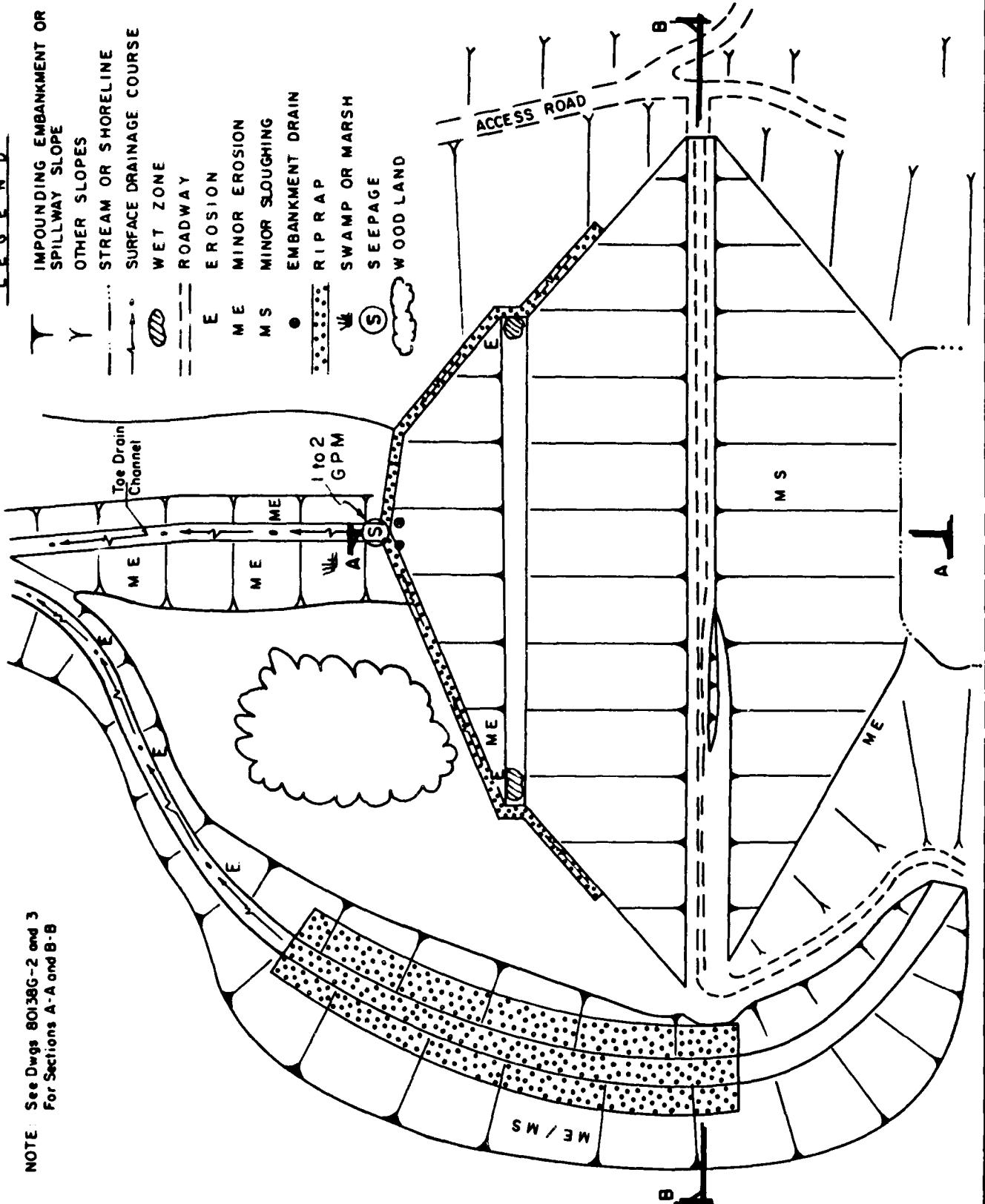
<u>VISUAL EXAMINATION OF</u>	<u>OBSERVATIONS</u>	<u>REMARKS OR RECOMMENDATIONS</u>
<b>CHANNEL (OBSTRUCTIONS, DEBRIS, ETC.)</b>	<p>The spillway discharge channel joins the toe drain channel approximately 700 feet below the dam in an area that was formerly the impoundment zone of Lake Calydon. Lake Calydon was removed prior to construction of Pond 6. Lake Calydon Dam was located approximately 400 feet below the toe of the Pond 6 embankment. Below this, the downstream channel passes through an uninhabited brush and tree-lined valley for approximately 1.3 miles where it joins the Center Branch of Pigeon Creek. In this reach, the Creek passes beneath three township roads and a Conrail railyard below the Mine No. 60 complex.</p>	
	<p>Approximately three miles below the dam, Center Branch joins Pigeon Creek. At 3.6 miles, Pigeon Creek enters a reservoir above the Ellsworth Mine No. 51 complex on the outskirts of Ellsworth, Pennsylvania.</p>	
<b>APPROXIMATE NUMBER OF HOMES AND POPULATION</b>		<p>In the first five miles below Pond 6 there are at least five inhabited dwellings that lie on the flood plain at elevations low enough to possibly be affected by high flows.</p>

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graph TD
    A[IMPOUNDING EMBANKMENT OR SPILLWAY SLOPE] -- T --> B[Y]
    B --> C[OTHER SLOPES]
    B --> D[STREAM OR SHORELINE]
    C --> E[WET ZONE]
    D --> F[ROADWAY]
    F --> G[E]
    F --> H[ME]
    F --> I[MS]
    G --> J[EMBANKMENT DRAIN]
    H --> J
    I --> J
    G --> K[RIP RAP]
    H --> K
    I --> K
    K --> L[SWAMP OR MARSH]
    L --> M[SEE PAGE]
    M --> N[WOOD LAND]
    E --> J
    J --> L
  
```

**NOTE:** See Dwg 80138G-2 and 3  
For Sections A-A and B-B



DATE:	MAY 1981
SCALE:	NONE
DR:	JF
CK:	JEB
DWG. NO. 80138G-1	

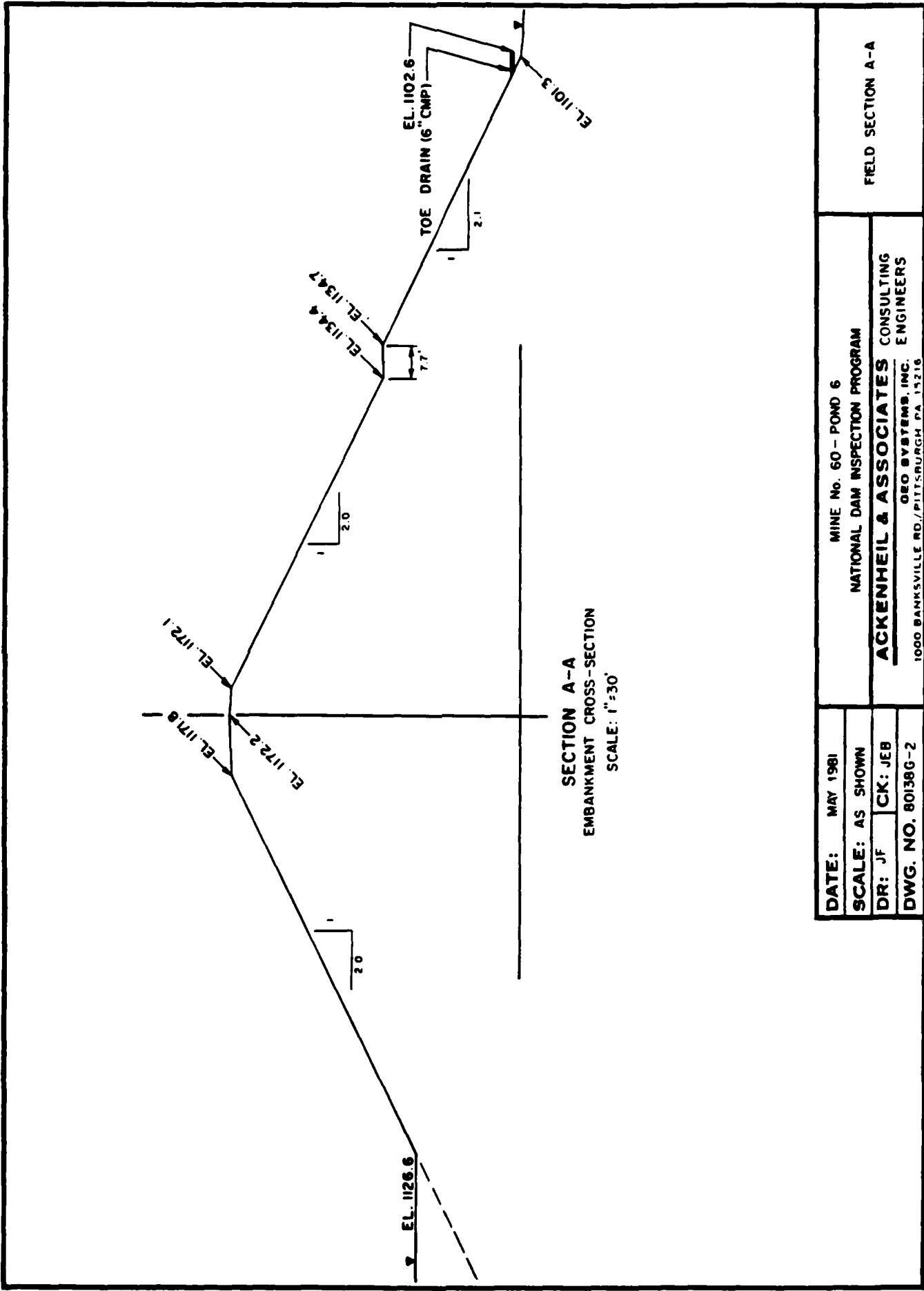
**MINE No. 60 - POND 6**  
**NATIONAL DAM INSPECTION PROGRAM**

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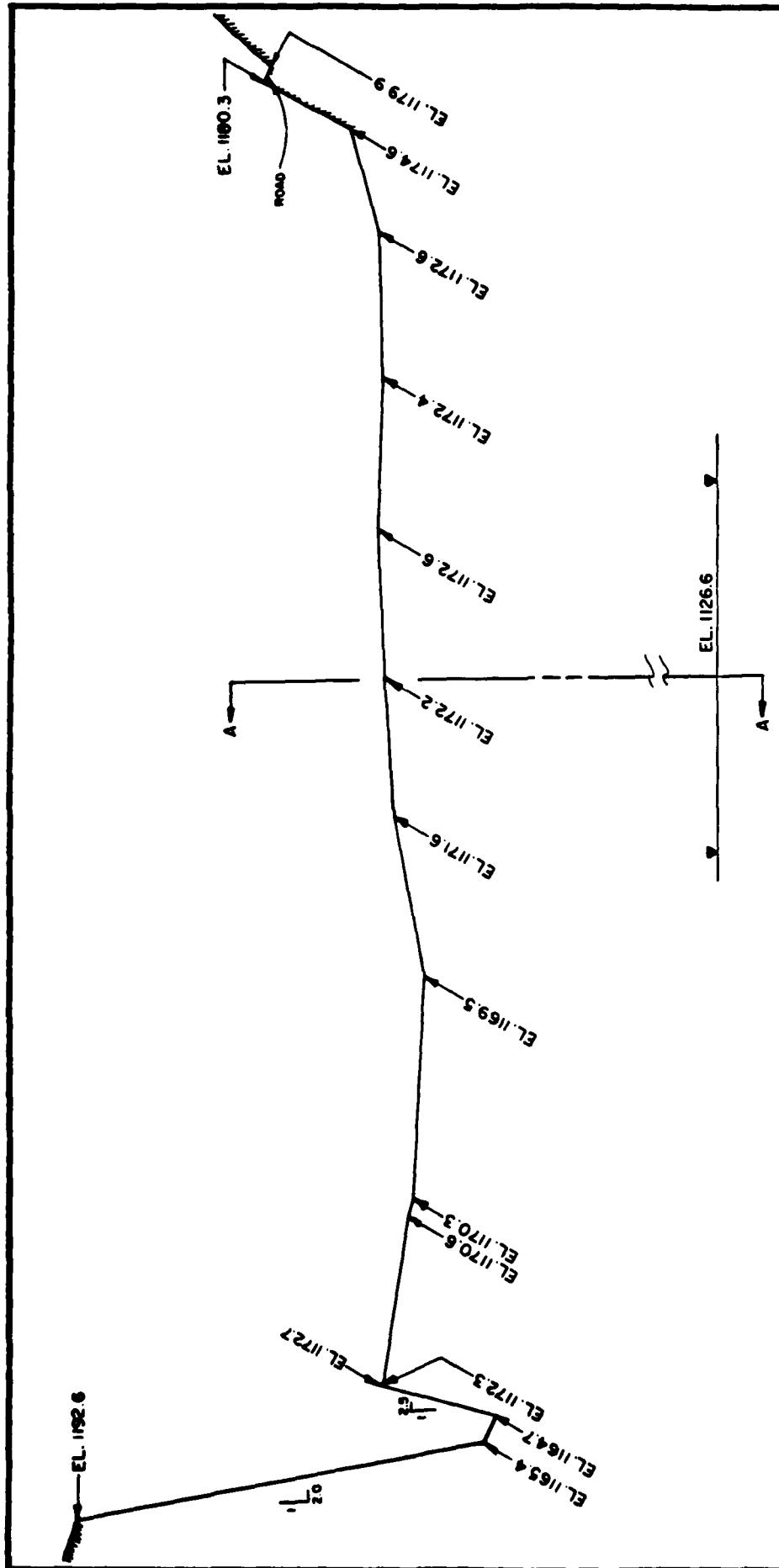
**ACKENHEIL & ASSOCIATES** CONSULTING  
**GEO SYSTEMS, INC.** ENGINEERS

100 BANKSVILLE RD./PITTSBURGH, PA 15216

## FIELD SKETCH



DATE: MAY 1981	MINE NO. 60 - POND 6	FIELD SECTION A-A
SCALE: AS SHOWN	NATIONAL DAM INSPECTION PROGRAM	
DR: JF	CK: JEB	<b>ACKENHEIL &amp; ASSOCIATES</b> CONSULTING
DWG. NO. 80138G-2		ODO SYSTEMS, INC. 1000 BANKSVILLE RD./PITTSBURGH PA 15216



**SECTION B-B**  
**DAM PROFILE**  
**(LOOKING DOWNSTREAM)**

SCALE:  
 HORZ. 1" = 100'  
 VERT. 1" = 10'

DATE: MAY 1981		MINE NO. 60 - POND 6		NATIONAL DAM INSPECTION PROGRAM		FIELD SECTION B-B	
SCALE: AS SHOWN		ACKENHEIL & ASSOCIATES		CONSULTING		GEO SYSTEMS, INC.	
DR: JF		CK: JEB		ENGINEERS		1000 BANKSVILLE RD / PITTSBURGH PA 15216	
DWG. NO. 80138G-3							

APPENDIX B  
ENGINEERING DATA CHECKLIST

CHECK LIST  
 ENGINEERING DATA  
 DESIGN, CONSTRUCTION, OPERATION  
 PHASE I

NAME OF DAM Mine No. 60-Pond 6  
 NDI No. PA 01145

ITEM	REMARKS
Design Drawings*	<p>Set of drawings entitled "Construction Drawings for Slurry Impoundment for Mine 60" prepared by Michael Baker, Jr. Inc. of Beaver, Pennsylvania, dated 1980, including:</p> <ul style="list-style-type: none"> <li>1 of 17 General Plan.</li> <li>** 2 of 17 General Plan, Test Pit and Test Boring Locations.</li> <li>** 3 of 17 General Plan, Test Pit and Test Boring Locations.</li> <li>** 4 of 17 Profile of Dam Base Line "D".</li> <li>** 5 of 17 Emergency Spillway.</li> <li>** 6 of 17 Exit Channel and Details.</li> <li>7 of 17 Existing Dam and Details.</li> <li>8 of 17 Geologic Sections.</li> <li>** 9 of 17 Cutoff Trench and Toe Drain Profile.</li> <li>** 10 of 17 Drainage Details.</li> <li>** 11 of 17 Test Pit and Boring Records.</li> <li>** 12 of 17 Stability Analyses and Triaxial Shear Results.</li> <li>13 of 17 Laboratory Soil Test and Filter Gradations.</li> <li>** 14 of 17 Construction Specifications.</li> <li>15 of 17 Cross Sections Base Line "A".</li> <li>16 of 17 Cross Sections Base Line "A".</li> <li>17 of 17 Cross Sections Base Line "A".</li> </ul>
As-Built Drawings	None available.

ITEM	REMARKS
Regional Vicinity Map	U.S.G.S. 7-1/2 minute Hacket, Pennsylvania Quadrangle Map.
Construction History***	Built September through December 1980; to be completed 1981. Construction by Mack Equipment Corporation, Eighty-Four, Pennsylvania.
Typical Sections of Dam*	See Design Drawings above.
Outlets-Plan	None available.
Details	
Constraints	
Discharge Ratings	
Rainfall/Reservoir Records	None available.
Design Reports	None available.
Geology Reports*	See Design Drawings above.
Design Computations	None available.
Hydrology and Hydraulics	None available.
Dam Stability*	See Design Drawings above.

ITEM	REMARKS
Seepage Studies	None available.
Materials Investigations, * Boring Records, Laboratory, Field	See Design Drawings above.
Post-Construction Surveys of Dam	None available.
Borrow Sources**	On site.
Monitoring Systems	None available.
Modifications	None reported.
High Pool Records	None reported.
Post-Construction Engineering Studies and Reports	None available.
Maintenance, Operation, Records	None available.
Spillway - Plan* Sections Details	See Design Drawings above.

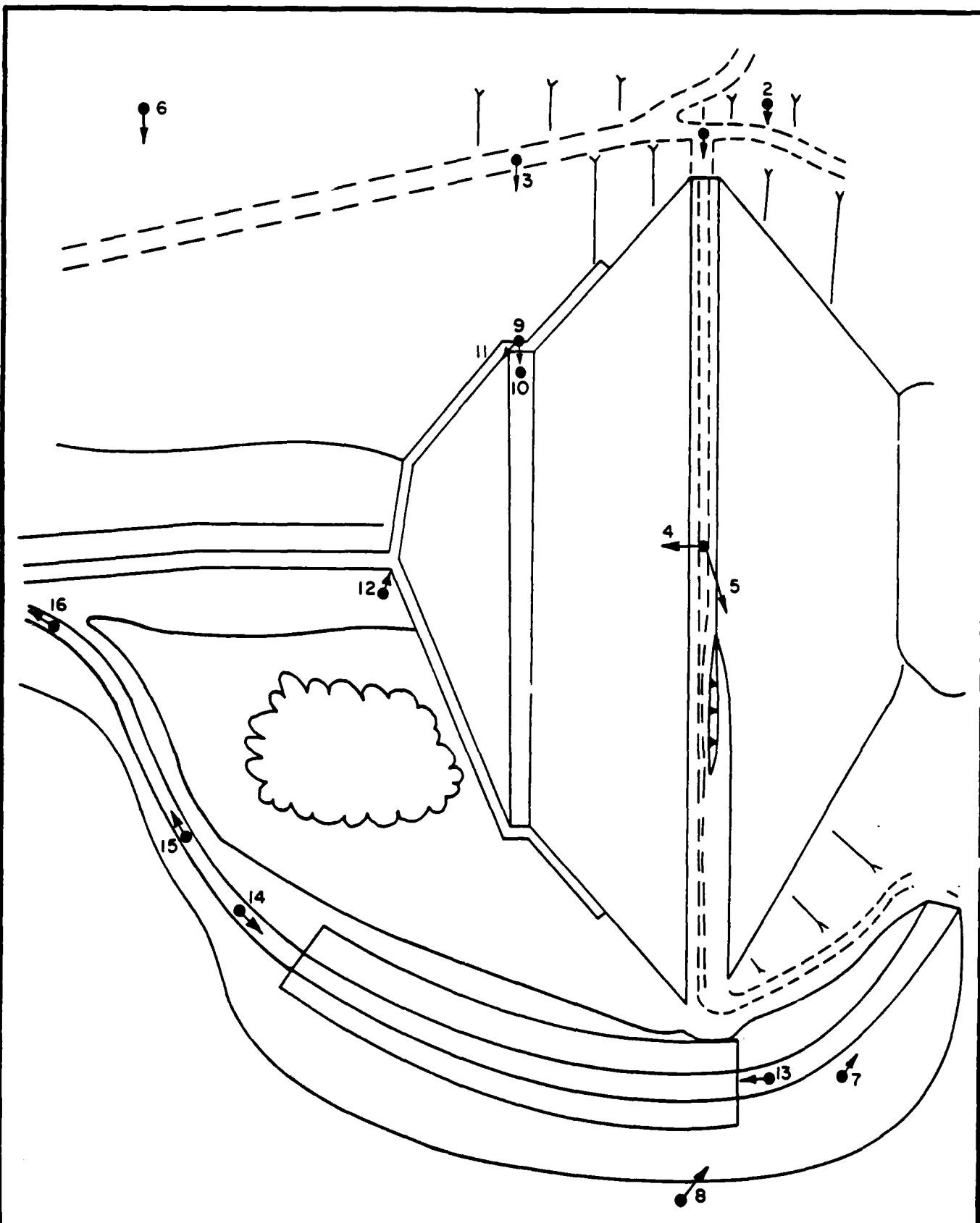
<u>ITEM</u>	<u>REMARKS</u>
Operating Equipment Plans and Details	None available.
Specifications*	See Design Drawings above.
Miscellaneous	No additional information available.
Prior Accidents or Failure of Dam, Description Reports	None reported.

\*Drawings obtained from the Pennsylvania Department of Environmental Resources.

\*\*Reduced size reproductions contained in Appendix E.

\*\*\*Information obtained from Bethlehem Mines Corporation, Ellsworth-Butler Division.

APPENDIX C  
PHOTOGRAPHS



DATE: MAY 1981  
SCALE: NONE  
DR: JF CK: JEB  
DWG. NO. 60138G

MINE No. 60 - POND 6  
NATIONAL DAM INSPECTION PROGRAM  
ACKENHEIL & ASSOCIATES CONSULTING  
ENGINEERS  
GEO SYSTEMS, INC.  
1900 BANKSVILLE RD / PITTSBURGH PA 15216

PHOTO KEY MAP

MINE NO 60 - POND 6



MINE NO 60 - POND 6

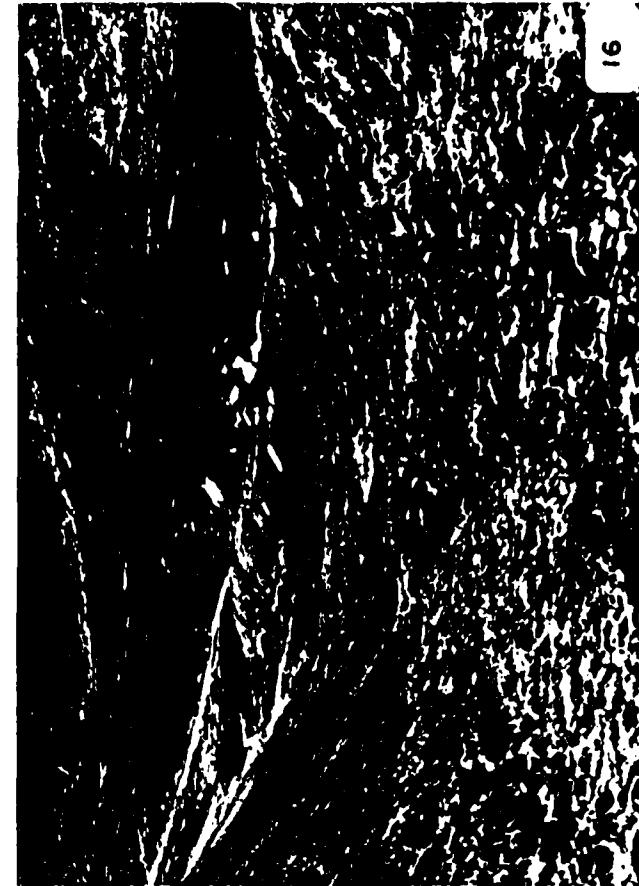


C3

MINE NO. 60 - POND 6



MINE NO 60 - POND 6



C5

#### PHOTO DESCRIPTIONS

- Photo 1 Embankment Crest Overview.
- Photo 2 Upstream Slope showing erosion in foreground.
- Photo 3 Downstream Slope showing bench and toe area.
- Photo 4 Overview of Downstream Channel.
- Photo 5 Overview of Principal (and Emergency) Spillway Approach Channel on opposite shore.
- Photo 6 Overview of Principal (and Emergency) Spillway Channel.
- Photo 7 Principal (and Emergency) Spillway Entrance looking upstream.
- Photo 8 Spillway Channel Erosion Protection Blanket.
- Photo 9 Overview of Bench on Downstream Slope showing wet zones.
- Photo 10 Close Up of Bench.
- Photo 11 Riprapped Left Groin.
- Photo 12 Foundation Drain Outlets at Toe of Embankment.
- Photo 13 Principal (and Emergency) Spillway Channel, beginning of riprap blanket.
- Photo 14 Principal (and Emergency) Spillway Channel, end of riprap blanket.
- Photo 15 Spillway Discharge Channel.
- Photo 16 Intersection of Spillway Discharge Channel and Toe Drain Channel.

APPENDIX D  
HYDROLOGY AND HYDRAULICS  
ANALYSES

APPENDIX D  
HYDROLOGY AND HYDRAULICS  
ANALYSES

Methodology: The dam overtopping analysis was accomplished using the systemized computer program HEC-1 (Dam Safety Version), July 1978, prepared by the Hydrologic Engineering Center, U.S. Army Corps of Engineers, Davis, California. A brief description of the methodology used in the analysis is presented below.

1. Precipitation: The Probable Maximum Precipitation (PMP) is derived and determined from regional charts prepared from past rainfall records including "Hydrometeorological Report No. 33" prepared by the U.S. Weather Bureau.

The index rainfall is reduced from 10% to 20% depending on watershed size by utilization of what is termed the HOP Brook adjustment factor. Distribution of the total rainfall is made by the computer program using distribution methods developed by the Corps.

2. Inflow Hydrograph: The hydrologic analysis used in development of the overtopping potential is based on applying a hypothetical storm to a unit hydrograph to obtain the inflow hydrograph for reservoir routing.

The unit hydrograph is developed using the Snyder method. This method requires calculation of several key parameters. The following list gives these parameters, their definition and how they were obtained for these analyses.

<u>Parameter</u>	<u>Definition</u>	<u>Where Obtained</u>
C <sub>t</sub>	Coefficient representing variations of watershed	From Corps of Engineers*
L'	Length from centroid of watershed to spillway	From U.S.G.S. 7.5 minute topographic map
C <sub>p</sub>	Peaking coefficient	From Corps of Engineers*
A	Watershed size	From U.S.G.S. 7.5 minute topographic map

3. Routing: Reservoir routing is accomplished by using Modified Puls routing techniques where the flood hydrograph is routed through reservoir storage. Hydraulic capacities of the outlet works, spillways and the crest of the dam are used as outlet controls in the routing.

The hydraulic capacity of the outlet works can either be calculated and input or sufficient dimensions input and the program will calculate an elevation-discharge relationship.

Storage in the pool area is defined by an area-elevation relationship from which the computer calculates storage. Surface areas are either planimetered from available mapping or U.S.G.S. 7.5 minute series topographic maps or taken from reasonably accurate design data.

4. Dam Overtopping: Using given percentages of the PMF the computer program will calculate the percentage of the PMF which can be controlled by the reservoir and spillway without the dam overtopping.

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\*Developed by the Corps of Engineers on a regional basis for Pennsylvania.

HYDROLOGIC AND HYDRAULIC  
ENGINEERING DATA

DRAINAGE AREA CHARACTERISTICS: Predominately woods and  
brushland. No major development noted.

ELEVATION-TOP NORMAL POOL (STORAGE  
CAPACITY): Varies (varies) Maximum 1165.4 (525 acre-feet)

ELEVATION-TOP FLOOD CONTROL POOL (STORAGE  
CAPACITY): 1165.4 (525 acre-feet)

ELEVATION-MAXIMUM DESIGN POOL: 1172.0

ELEVATION-TOP DAM: 1169.5 (on inspection date)

OVERFLOW SECTION

- a. Elevation Varies from 1164.7 to 1165.4
- b. Type Broad crested weir in trapezoidal open channel
- c. Width 30 feet
- d. Length 20 feet
- e. Location Spillover Left Abutment
- f. Number and Type of Gates None

OUTLET WORKS AND DRAWDOWN FACILITY

- a. Type Not installed at time of inspection
- b. Location Right reservoir slope
- c. Entrance Invert N/A
- d. Exit Inverts N/A
- e. Emergency Drawdown Facilities None

HYDROMETEOROLOGICAL GAGES

- a. Type None
- b. Location N/A
- c. Records None

MAXIMUM REPORTED NON-DAMAGING  
DISCHARGE None reported

HEC-1 DAM SAFETY VERSION  
HYDROLOGY AND HYDRAULIC ANALYSIS  
DATA BASE

NAME OF DAM: Mine No. 60-Pond 6	NDI NO. PA 01145
Probable Maximum Precipitation (PMP)	24.2*
Drainage Area	0.16 sq. mi.
Reduction of PMP Rainfall for Data Fit Reduce by 20%, therefore PMP rainfall	0.8 (24.2) =19.4 inches
Adjustments of PMF for Drainage Area (Zone 7)	
6 hrs.	102%
12 hrs.	120%
24 hrs.	130%
48 hrs.	140%
Snyder Unit Hydrograph Parameters (Pond Full Condition)	
Zone	29**
C <sub>p</sub>	0.5
C <sub>t</sub>	1.6
L'	0.19 mile
t <sub>p</sub> = C <sub>t</sub> (L') <sup>0.6</sup> =	0.59 hours
Loss Rates	
Initial Loss	1.0 inch
Constant Loss Rate	0.05 inch/hour
Base Flow Generation Parameters	
Flow at Start of Storm	1.5 cfs/sq.mi=0.24 cfs
Base Flow Cutoff	0.05 x Q <sub>peak</sub>
Recession Ratio	2.0
Overflow Section Data	
Crest Length	20 feet
Crest Elevation	1165.4
Freeboard (Pond Full Condition, Current Embankment Crest Elevation)	4.1 feet
Discharge Coefficient	3.09
Exponent	1.5
Discharge Capacity	513 cfs

\* Hydrometeorological Report 33

\*\*Hydrological zone defined by Corps of Engineers, Baltimore  
District, for determining Snyder's Coefficients (C<sub>p</sub> and C<sub>t</sub>).

ACKENHEIL & ASSOCIATES  
GEO Systems, Inc.  
1000 Banksville Road  
PITTSBURGH, PA 15216  
(412) 531-7111

Job MINEGO POND 6 Job No 80385  
Subject DATA INPUT  
Mode By JPF Date 4/1/81 Checked JEB Date 4/20/81

### Loss Rate and Base Flow Parameters

As Recommended By Corps of Engineers, Baltimore District

STRTL = 1 INCH  
CNSTL = 0.05 IN/HR  
STRTQ = 1.5 cfs/mi<sup>2</sup>  
QRCSEN = 0.05 (5% of Peak Flow)  
RTIOR = 2.0

### Elevation - Area - Capacity Relationships

From USGS 7.5 minute Quad, Owners information, and  
field inspection data

AT Elevation 1165.4      AREA = 21.1 ACRES  
Storage = 525 ACRE-FT.

AT Elevation 1180      AREA = 29.4 ACRES

From the CONIC METHOD of Reservoir Volume  
Flood Hydrograph Package (HEC-1)  
Dam Safety Version (Users Manual)

$$H = 3V/A = \frac{3(525)}{21.1} = 74.6 \text{ feet}$$

Elevation where Area Equals 200

$$1165.4 - 74.6 = 1090.8$$

AREA Elevation	\$A \$E	0. 1090.8	21.1 1165.4	29.4 1180.0
-------------------	------------	--------------	----------------	----------------

ACKENHEIL & ASSOCIATES  
GEO Systems, Inc.  
1000 Banksville Road  
PITTSBURGH, PA 15216  
(412) 531-7111

Job MINEGO POND 6 Job No 801380  
Subject DATA INPUT  
Mode By JPA Date 4/18/checked JBB Date 4/26/81

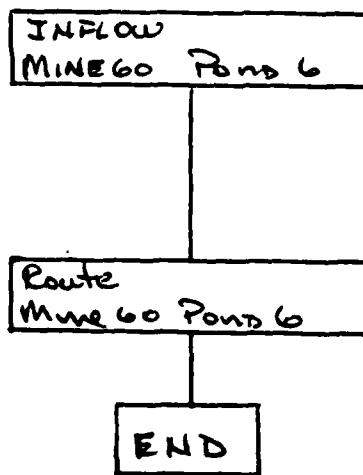
### OVERTOP Parameters

Top of Dam Elevation (minimum)	1169.5
Length of Dam	845. feet
Coefficient of Discharge	3.09

### SPILLWAY PARAMETERS

CREST Elevation	1165.4
CREST Length	20.0 feet
Coefficient of Discharge	3.09
SIDE Slopes	Assumed Rectangular

### Program Schedule



D6

JOB 7936

FLOOD HYDROGRAPH PACKAGE (HEC-1)  
DAM SAFETY VERSION JULY 1978  
LAST MODIFICATION 26 FEB 79

NATIONAL PROGRAM FOR THE INSPECTION OF NON FEDERAL DAMS  
HYDROLOGIC AND HYDRAULIC ANALYSIS OF SLURRY IMPOUNDMENT #6  
PROBABLE MAXIMUM FLOOD PMF/UNIT HYDROGRAPH BY SNYDER'S METHOD

1 A1  
2 A2  
3 A3  
4 B 300 0 10 0 0 0 0 0 -4 0  
5 B1 5  
6 J 1 2 1  
7 J1 1. .5  
8 K 0 1 1  
9 K1 INFLOW HYDROGRAPH FOR SLURRY IMPOUNDMENT #6, MINE 60  
10 M 1 1 0.16 0.16 1  
11 P 24.2 102 120 130 140  
12 T 1.0 .05  
13 W 0.59 0.5  
14 X -1.5 -0.05 2.0  
15 K 1 2 1  
16 K1 ROUTING AT SLURRY IMPOUNDMENT #6, MINE 60  
17 Y 1 1 525.  
18 Y1 1  
19 SA 0. 21.1 29.4  
20 \$E1090.8 1165.4 1180.  
21 \$S1165.4 20.0 3.09 1.5  
22 \$D1169.5 3.09 1.5 845.  
23 K 99  
24 A  
25 A  
26 A  
27 A  
28 A

PREVIEW OF SEQUENCE OF STREAM NETWORK CALCULATIONS

RUNOFF HYDROGRAPH AT 1  
ROUTE HYDROGRAPH TO 2  
END OF NETWORK

FLOOD HYDROGRAPH PACKAGE (HEC-1)  
DAM SAFETY VERSION JULY 1978  
LAST MODIFICATION 26 FEB 79

RUN DATE: 27 MAR 81  
RUN TIME: 14.43.22

NATIONAL PROGRAM FOR THE INSPECTION OF NON FEDERAL DAMS  
HYDROLOGIC AND HYDRAULIC ANALYSIS OF SLURRY IMPOUNDMENT #6  
PROBABLE MAXIMUM FLOOD PMF/UNIT HYDROGRAPH BY SNYDER'S METHOD

JOB SPECIFICATION										
NQ	NHR	NMIN	IDAY	IHR	IMIN	METRC	IPLT	IPRT	NSTAN	
300	0	10	0	0	0	0	0	0	-4	0
			JOPER	NWT	LROPT	TRACE				
			5	0	0	0				

MULTI-PLAN ANALYSES TO BE PERFORMED  
NPLAN= 1 NRTIO= 2 LRTIO= 1  
RTIOS= 1.00 0.50

D7

## SUB-AREA RUNOFF COMPUTATION

## INFLOW HYDROGRAPH FOR SLURRY IMPOUNDMENT #6, MINE 60

ISTAQ	ICOMP	IECON	ITAPE	JPLT	JPRT	I NAME	I STAGE	IAUTO
1	0	0	0	0	0	1	0	0

HYDROGRAPH DATA									
IHYDG	IUNG	TAREA	SNAP	TRSDA	TRSPC	RATIO	ISNOW	ISAME	LOCAL
1	1	0.16	0.0	0.16	0.0	0.0	0	1	0

PRECIP DATA									
SPFE	PMS	R6	R12	R24	R48	R72	R96		
0.0	24.20	102.00	120.00	130.00	140.00	0.0	0.0		

TRSPC COMPUTED BY THE PROGRAM IS 0.800

LOSS DATA										
LROPT	STRKR	DLTKR	RTIOL	ERAIN	STRKS	RTICK	STRIL	CNSTL	ALSMX	RTIMP
0	0.0	0.0	1.00	0.0	0.0	1.00	1.00	0.05	0.0	0.0

UNIT HYDROGRAPH DATA									
TP=	0.59	CP=0.50	NTA=	0					

RECEDITION DATA									
STPTQ=	-1.50	QRCSN=	-0.05	RTIOR=	2.00				

UNIT HYDROGRAPH 28 END-OF-PERIOD ORDINATES, LAG= 0.59 HOURS, CP= 0.50 VOL= 1.00

11.	41.	72.	85.	76.	62.	50.	41.	33.	27.
22.	18.	15.	12.	10.	8.	7.	5.	4.	4.
3.	2.	2.	2.	1.	1.	1.	1.	1.	

END-OF-PERIOD FLOW													
MD.DA	HR.MN	PERIOD	RAIN	EXCS	LOSS	COMP Q	MD.DA	HR.MN	PERIOD	RAIN	EXCS	LOSS	COMP Q
0													

SUM	27.10	24.68	2.42	15282.
( 688.)	( 627.)	( 61.)	( 432.74	

\*\*\*\*\* \*\*\*\*\* \*\*\*\*\* \*\*\*\*\* \*\*\*\*\*

## HYDROGRAPH ROUTING

## ROUTING AT SLURRY IMPOUNDMENT #6, MINE 60

ISTAQ	ICOMP	IECON	ITAPE	JPLT	JPRT	I NAME	I STAGE	IAUTO
2	1	0	0	0	0	1	0	0

ROUTING DATA									
CLOSS	CLOSS	Avg	IRES	ISAME	IOPT	IPMP	LSTR		

0.0	0.0	0.0	1	1	0	0	0		
NSTPS	NSTDL		LAG	AMSKK	X	TSK	STORA	ISPRAT	
1	0		0	0.0	0.0	0.0	525.	0	

SURFACE AREA= 0. 21. 29.

CAPACITY= 0. 525. 892.

ELEVATION= 1091. 1165. 1180.

CREL	SPWID	COQW	EXPW	ELEV	COQL	CAREA	EXPL
1165.4	20.0	3.1	1.5	0.0	0.0	0.0	0.0

DAM DATA			
TOPEL	COQD	EXPD	DAMWID
1169.5	3.1	1.5	845.

PEAK OUTFLOW IS 376. AT TIME 41.33 HOURS

PEAK OUTFLOW IS 170. AT TIME 41.50 HOURS

\*\*\*\*\* \*\*\*\*\* \*\*\*\*\* \*\*\*\*\* \*\*\*\*\*

PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS  
 FLOWS IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)  
 AREA IN SQUARE MILES (SQUARE KILOMETERS)

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO FLOWS	
				RATIO 1 1.00	RATIO 2 0.50
HYDROGRAPH AT	1 (	0.16 0.41)	1 (	652. 18.47)(	326. 9.24)(
ROUTED TO	2 (	0.16 0.41)	1 (	376. 10.63)(	170. 4.81)(

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1 .....	ELEVATION	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM			
		1165.41	1165.40	1169.50			
	STORAGE	525.	525.	616.			
	OUTFLOW	0.	0.	513.			
RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	1168.73	0.0	598.	376.	0.0	41.33	0.0
0.50	1167.36	0.0	567.	170.	0.0	41.50	0.0

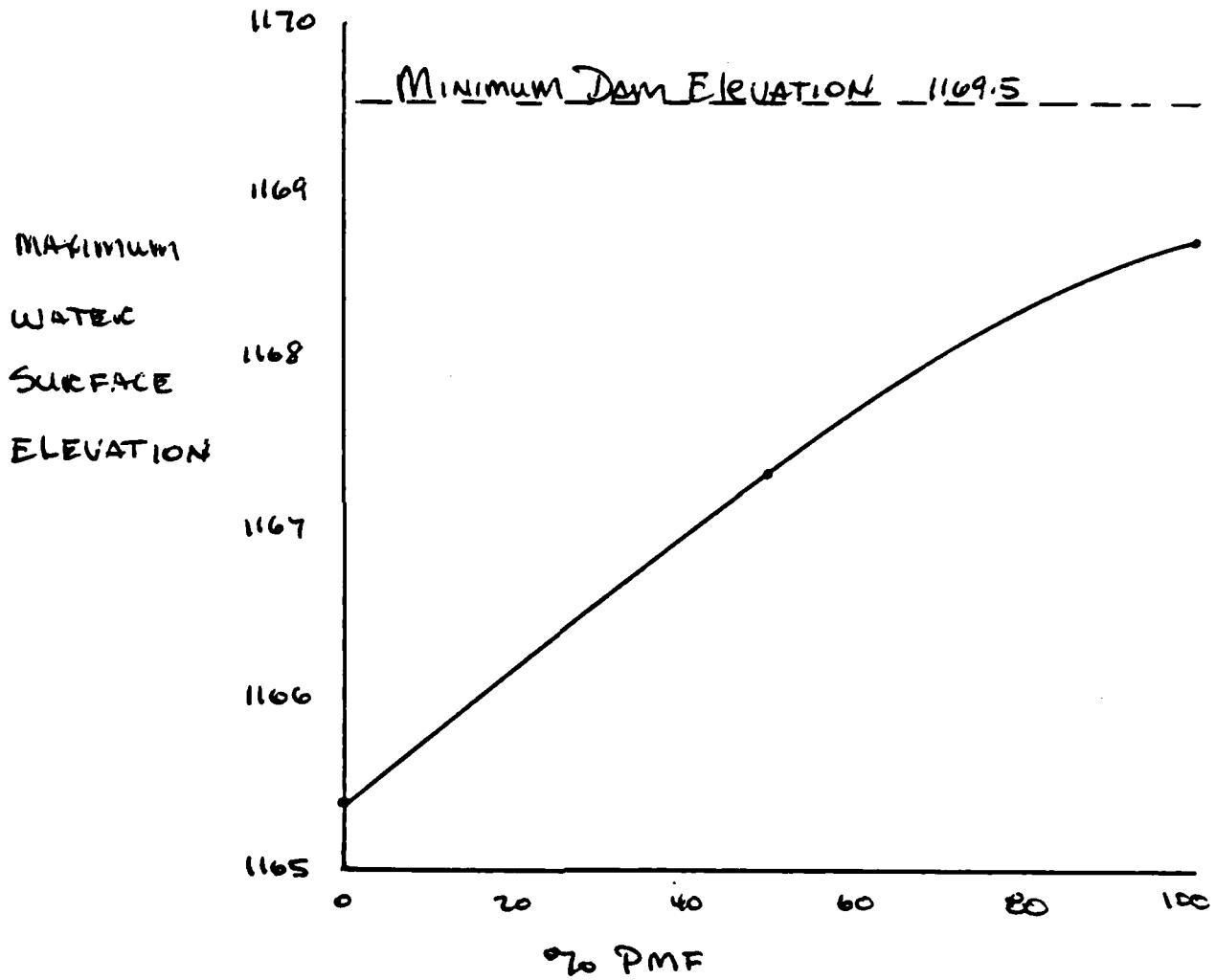
ACKENHEIL & ASSOCIATES  
GEO Systems, Inc.  
1000 Banksville Road  
PITTSBURGH, PA 15216  
(412) 531-7111

Job MINEGO Pond 6

Job No 501380

Subject HYDROLOGIC PERFORMANCE PLOT

Made By JDH Date 4/1/81 Checked JEB Date 4/2/81



D10

APPENDIX E

PLATES

LIST OF PLATES

Plate I           Regional Vicinity Map.

Plate II          Sheet 2 of 17, General Plan, Test Pit and Test Boring Locations.

Plate III         Sheet 3 of 17, General Plan, Test Pit and Test Boring Locations.

Plate IV         Sheet 4 of 17, Slurry Impoundment, Profile of Dam Base Line "D".

Plate V          Sheet 5 of 17, Spillway Impoundment, Emergency Spillway.

Plate VI         Sheet 6 of 17, Slurry Impoundment, Exit Channel and Details.

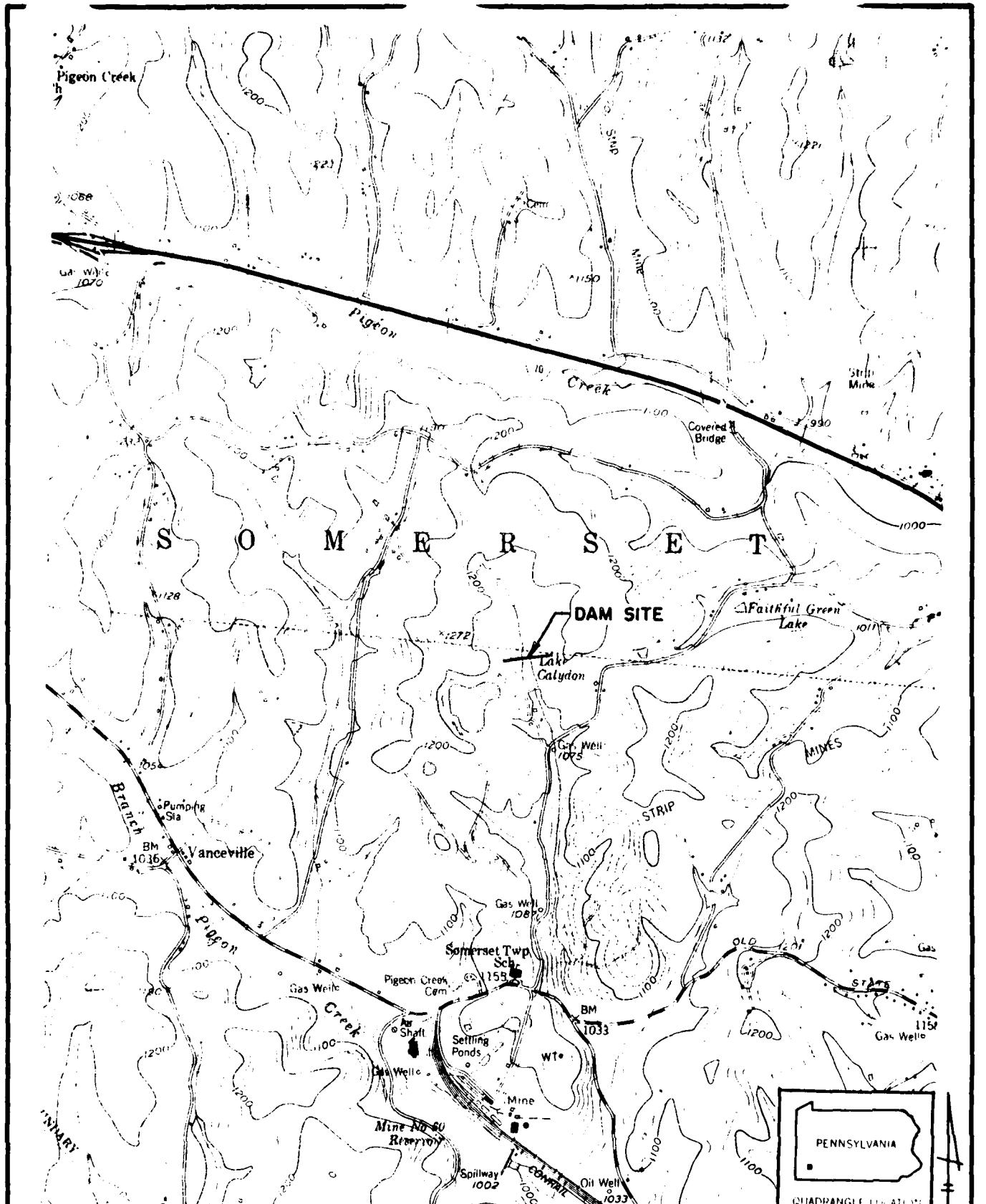
Plate VII        Sheet 9 of 17, Slurry Impoundment, Cutoff Trench and Toe Drain Profile.

Plate VIII       Sheet 10 of 17, Slurry Impoundment, Drainage Details.

Plate IX         Sheet 11 of 17, Test Pit and Boring Records.

Plate X          Sheet 12 of 17, Stability Analyses and Triaxial Shear Results.

Plate XI         Sheet 14 of 17, Construction Specifications.



HACKETT U.S.G.S. 7 1/2 min. QUADRANGLE

DATE: MAY 1981

SCALE: 1:2000

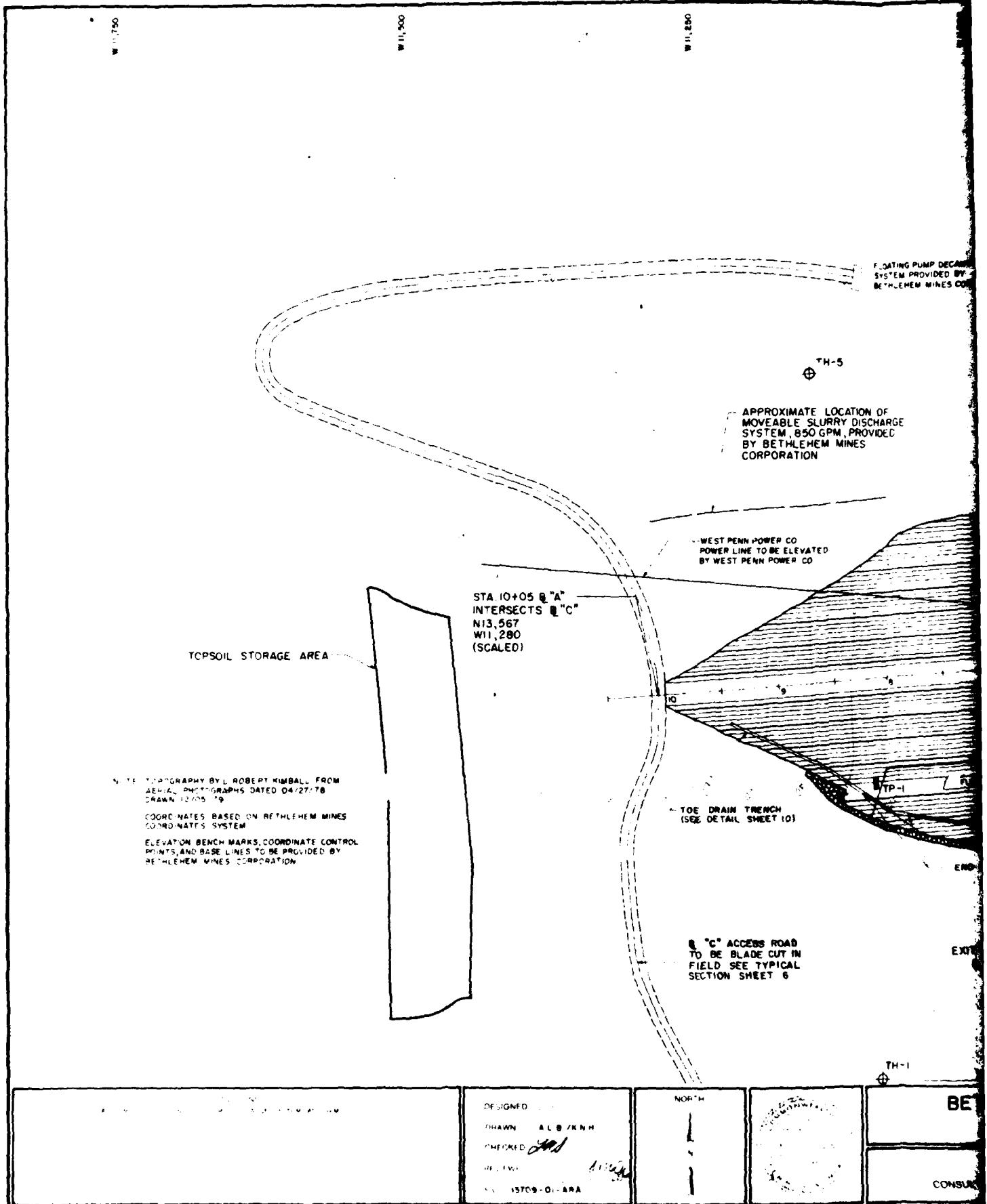
DR: JF CK

PLATE I

MINE No. 60 - POND 6  
NATIONAL DAM INSPECTION PROGRAM

**ACKENHEIL & ASSOCIATES** CONSULTING  
GEO SYSTEMS, INC. ENGINEERS  
1000 BANKSVILLE RD / PITTSBURGH, PA 15216

**REGIONAL  
VICINITY  
MAP**



11888

10,750

四百三

52

TH-6

WEST PENN POWER CO  
POWER LINE TO BE RELOCATED  
BY WEST PENN POWER CO

PC = +74  
 PT = 2+86  
 R = 30  
 L = 67+30  
 T = 45  
 C = 42  
 L.C = 967  
 P.I = 2+30

— 1140 —  
Q "A"  
N 13, 627  
W 10, 430

LEGEND  
FNU = GRADE CONTOUR  
BAS = LINE  
COORDINATE POINT (SCALED)

THE SERIES - DRIVE AND CORE BORINGS  
PERFORMED BY L. ROBERT KIMBALL &  
ASSOCIATES (DECEMBER 1979 AND  
JANUARY 1980)

TP SERIES - TEST PITS BY MICHAEL  
BAKER, JR., INC (FEBRUARY, 1980)

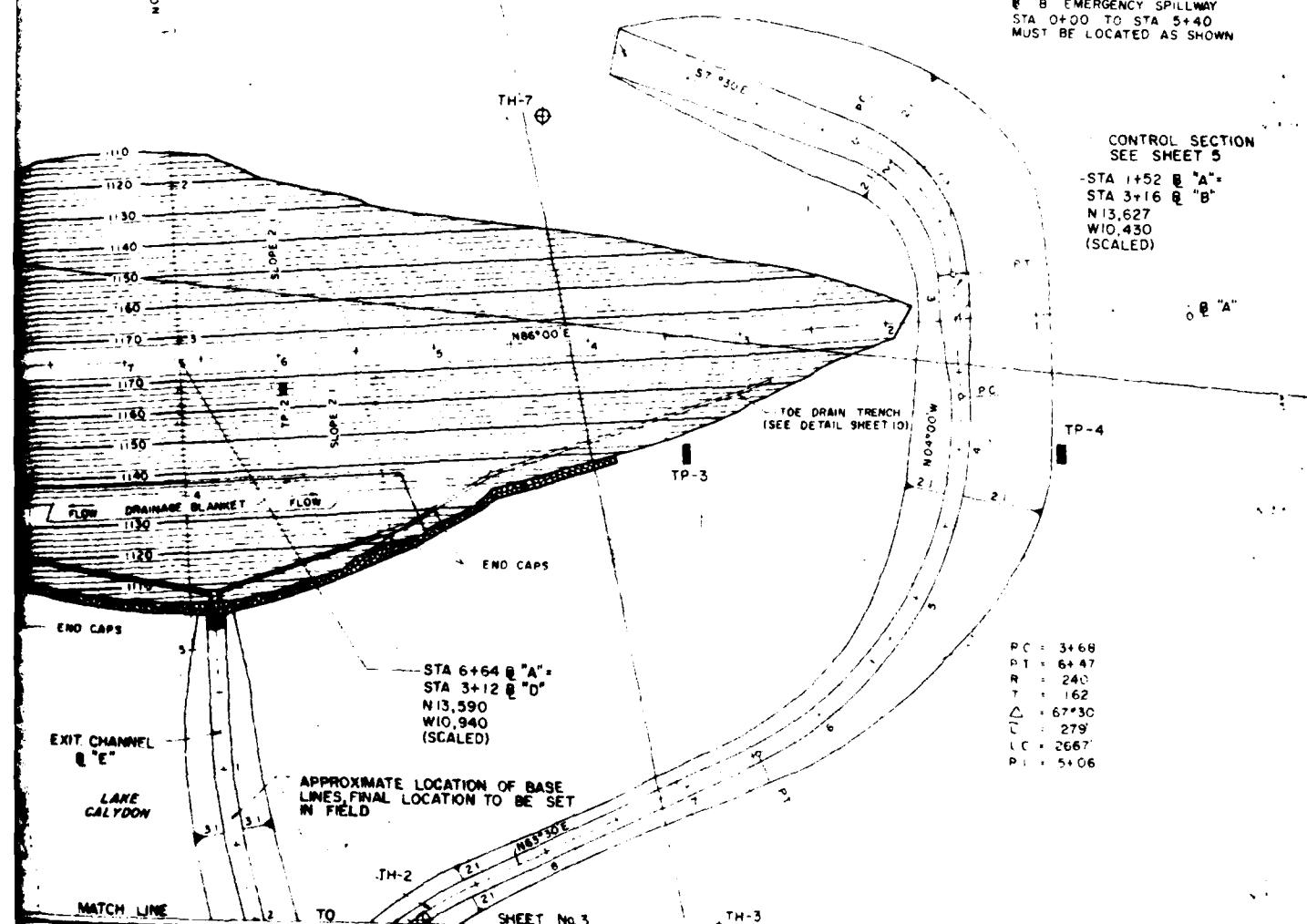
### NOTE

THE TH SERIES BORINGS ARE FIELD SURVEYED LOCATIONS THE TP SERIES ARE APPROXIMATE LOCATIONS ONLY.

EMERGENCY SPILLWAY @ "B"  
SEE SHEET 5

#### NOTE

¶ "B" EMERGENCY SPILLWAY  
STA 0+00 TO STA 5+40  
MUST BE LOCATED AS SHOWN



## BETHLEHEM MINES CORPORATION

MINE NO 60  
SLURRY IMPOUNDMENT

MICH  
CONSULTING ENGINEERS

## HEAVIER PENNIES PLEASE



# GENERAL PLAN

## TEST PIT & TEST BORING LOCATIONS

2

17

**PLATE II**

MATCH LINE

ABANDON

• "C" ACCESS ROAD  
TO BE BLADE CUT  
IN FIELD.  
SEE TYPICAL  
SECTION SHEET 6

EQUITABLE GAS 30  
3" GAS LINE

WARP ACCESS ROAD DRA  
SWALE TO MESH WITH  
EXISTING SWALE

NOTE TOPOGRAPHY BY L ROBERT KIMBALL FROM  
AERIAL PHOTOGRAPHS DATED 04/27/78  
DRAWN 12/05/79

COORDINATES BASED ON BETHLEHEM MINES  
COORDINATE SYSTEM

ELEVATION BENCH MARKS, COORDINATE  
CONTROL POINTS, AND BASE LINES TO  
BE PROVIDED BY BETHLEHEM MINES  
CORPORATION

#### LEGEND

— 1140 — FINISHED GRADE CONTOUR  
Q. "A" BASE LINE  
N 13, 627 COORDINATE POINT (SCALED)  
W 10, 430

TH SERIES - DRIVE AND CORE BORINGS  
PERFORMED BY L ROBERT KIMBALL &  
ASSOCIATES (DECEMBER 1979 AND  
JANUARY 1980)

TP SERIES - TEST PITS BY MICHAEL  
BAKER, JR., INC (FEBRUARY, 1980)

#### NOTE

THE TH SERIES BORINGS ARE FIELD  
SURVEYED LOCATIONS. THE TP SERIES  
ARE APPROXIMATE LOCATIONS ONLY

TEMPORARY ST  
HAY BALE ERO  
SEE SHEET 4

	DE-10001 DRAWN ALB 1980 BY W.H. RE-10001 3779 ST. LARA	NUMBER 1		BE1
				CONSUL

LINE TO SHEET No 2

STA. 10+44 R "B"  
STA. 3+28 R "E"  
N 13,108  
W 10,844  
(SCALED)

LAKE  
CAYRON

ONED

DAM BREACH  
SEE DETAILS  
SHEETS 6 & 7

EMERGENCY SPILLWAY  
@ "B"

APPROXIMATE LOCATION OF  
BASE LINES, FINAL LOCATION  
TO BE SET IN FIELD

STA 4 + 05 "E"  
STA 1 + 61 "F"  
N 13, 035  
W 10, 820  
(SCALED)

EXIT CHANNEL  
B "E"

WEST PENN POWER CO  
POWER LINE TO BE  
RELOCATED BY  
WEST PENN POWER CO

WASTE AREA FOR BREACH  
OF EXISTING DAM

TH-8

STRAW OR HAY BALE BARRIER  
60' LONG x 4 BALES WIDE x 4  
BALES HIGH (SEE SHEET 7 FOR  
DETAIL)

**ABANDONED**

## ABANDONED

## RY STRAW OR EROSION CONTROL

**BETHLEHEM MINES CORPORATION**  
MINE NO 60  
**SLURRY IMPOUNDMENT**

MICHAEL BAKER, JR., INC.

CONSULTING ENGINEERS REAVER, PENNSYLVANIA



## GENERAL PLAN

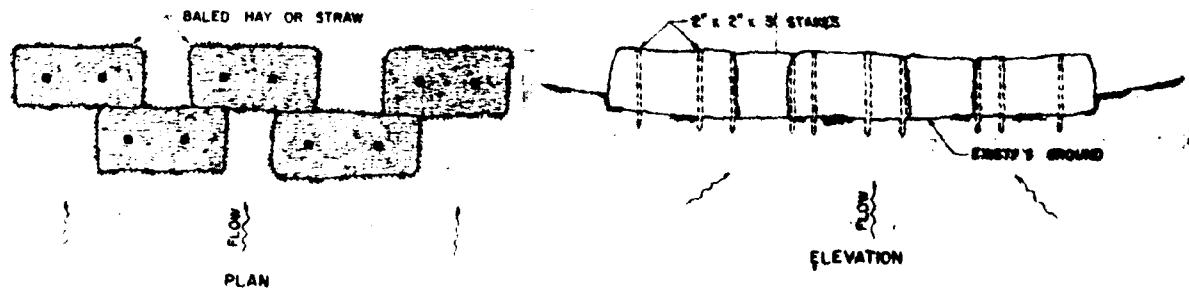
### TEST PIT & TEST BORING LOCATIONS

3

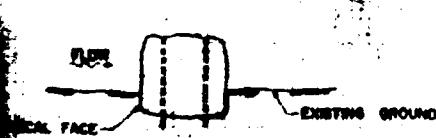
17

PLATE III

PROFILE OF DITCH



TEMPORARY STRAW OR HAY BALE EROSION CONTROL



EMBEDDING DETAIL

REVISIONS			BETHLEHEM MINES CORPORATION MINE NO. 60	
NO.	DATE	NAME	SLURRY IMPOUNDMENT PROFILE OF DAM BASE LINE "D"	
			SHEET 4 OF 17	S.O. 13709-01-ARA
			SCALE AS SHOWN	DATE MAY, 1980
			MICHAEL BAKER, JR. INC Consulting Engineers BEAVER PENNSYLVANIA	
			DESIGNED DRAWN / TRACED CHECKED FILE	

PLATE IV

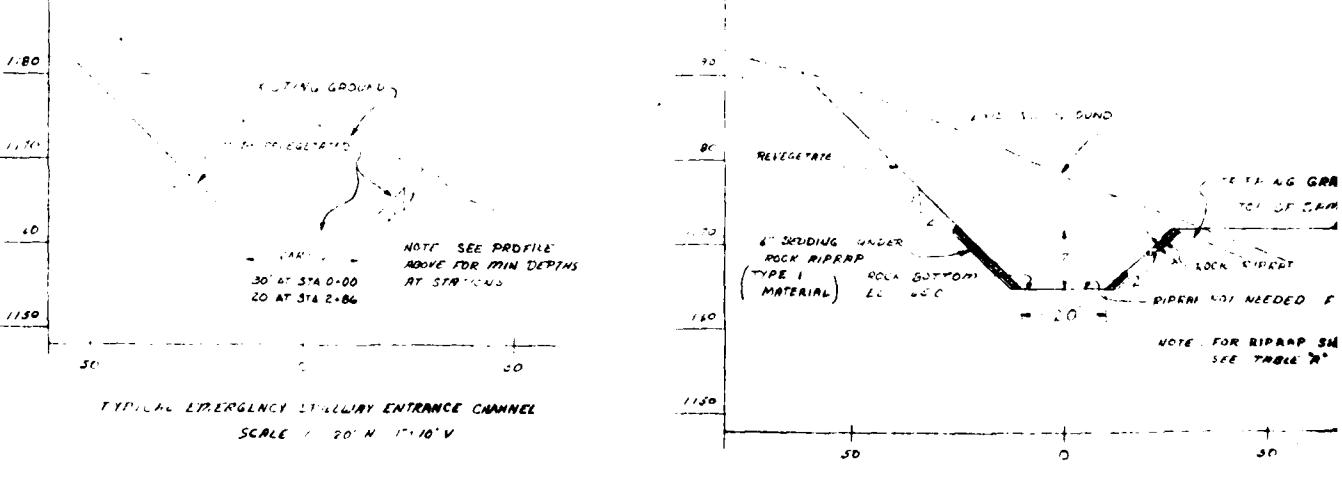
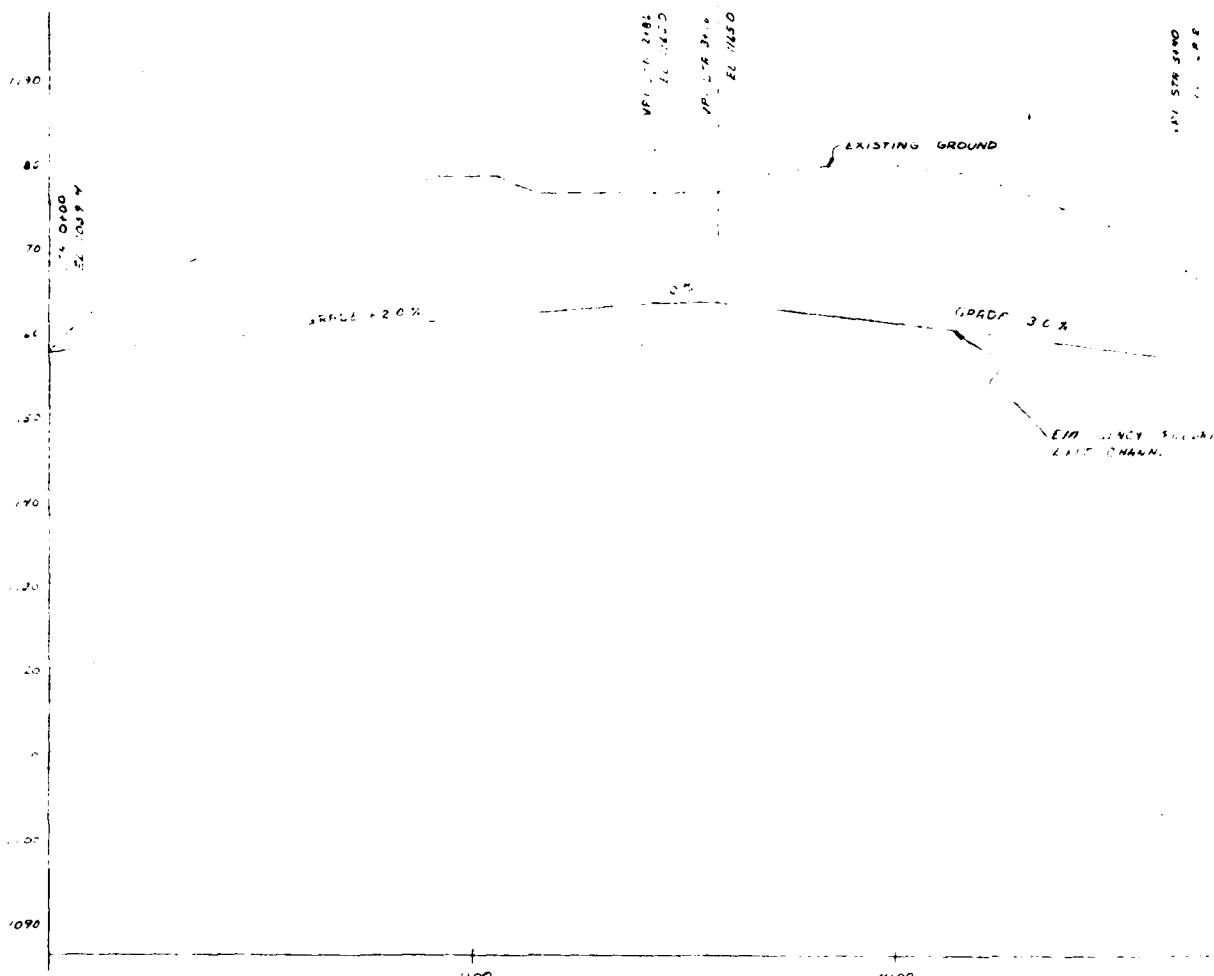
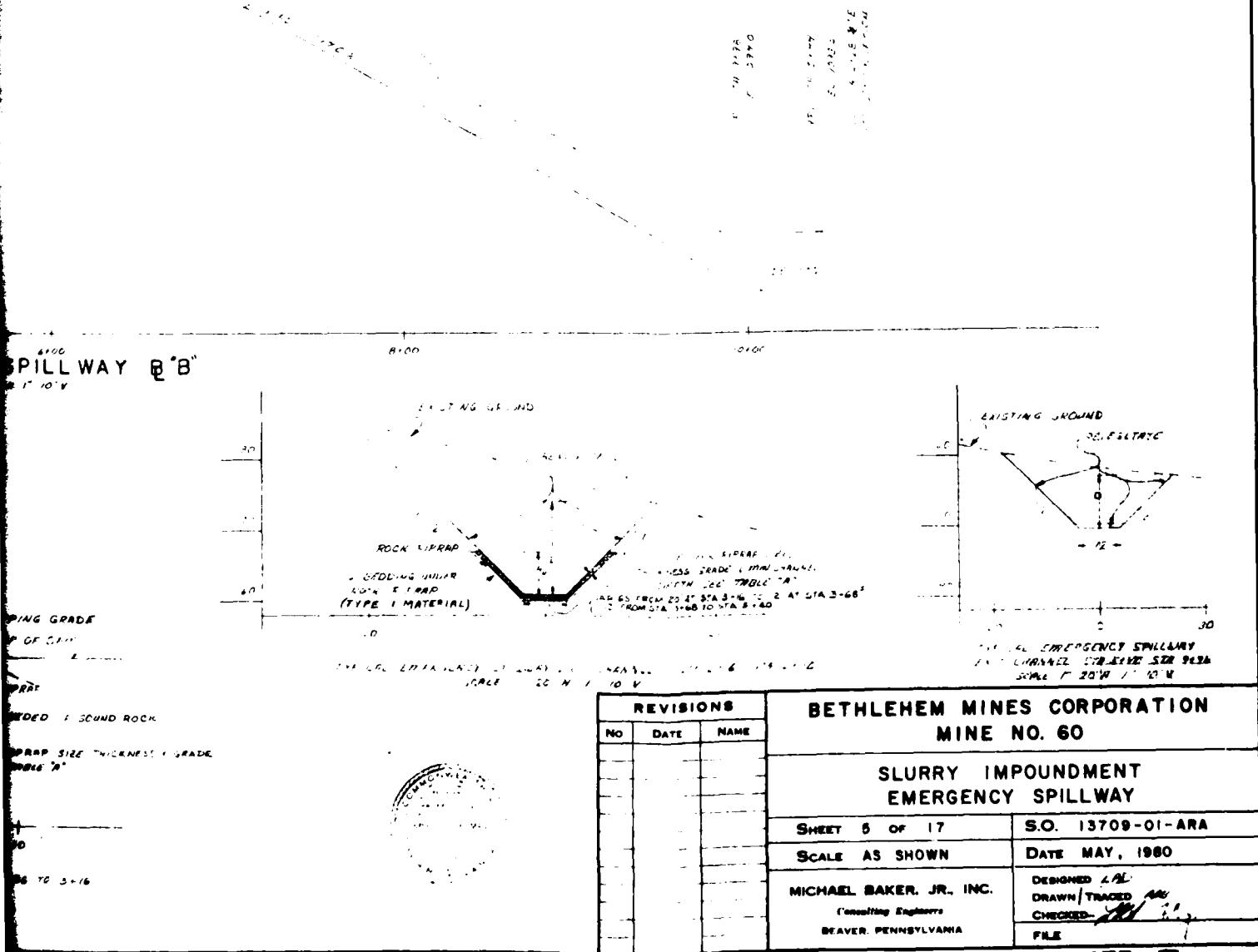


TABLE 'A'

STATION	DEPTH, FT	TEST	TEST	TEST	TEST
1000 000	00	200	200	200	200
1000 000	00	200	200	200	200
1000 000	00	200	200	200	200
1000 000	00	200	200	200	200



REVISIONS		
NO	DATE	NAME

**BETHLEHEM MINES CORPORATION  
MINE NO. 60**

**SLURRY IMPOUNDMENT  
EMERGENCY SPILLWAY**

SHEET 6 OF 17 S.O. 13709-01-ARA

**SCALE AS SHOWN**

**MICHAEL BAKER, JR., I**  
*Consulting Engineers*

**PLATE V**

FEEDING GROUNDS

EXIT CHANNEL HOT TOP

**PROFILE E**

5cm

1000 ft. above

### PLAYING GROUND

THE FEDERAL BUREAU  
OF INVESTIGATION  
U. S. DEPARTMENT OF JUSTICE

٤٧٠

OFFICIAL FRS CHANNEL 17  
SCALE 1" 20' H 1" 10' V

TYPE 6000 EMERGENCY THERMOMETER  
SCALE 32 to 100

30

### 1. EGETATM

EXISTING DRAWS

LINE OF EXISTING CHANNELS

300

100

FILE EXIT CHANNEL B/E

SCALE 1:20,000

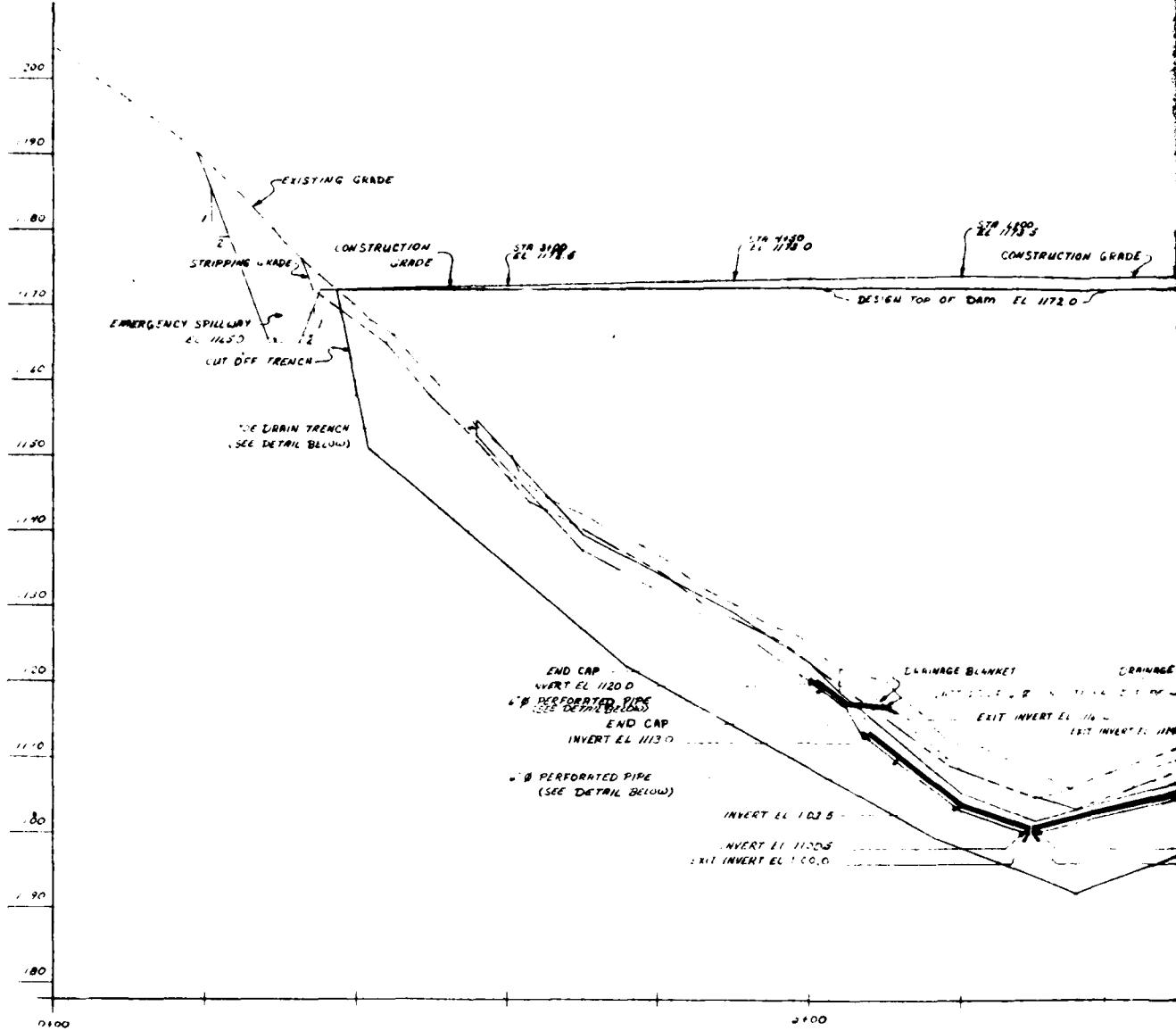
DETERMINED DEPTH  
FROM 100' TO 100' IN 60' DEPTH  
AND NO GEO INDIC.

SLURRY ACCESS ROAD B/E  
ONE 10' WIDE NO V

REVISIONS			BETHLEHEM MINES CORPORATION MINE NO. 60	
NO	DATE	NAME	SLURRY IMPOUNDMENT EXIT CHANNEL AND DETAILS	
			SHEET 8 OF 17	S.O. 13709-01-ARA DATE MAY, 1980
			SCALE AS SHOWN	
MICHAEL BAKER, JR., INC. Consulting Engineers BEAVER, PENNSYLVANIA		DESIGNED BY DRAWN/TRADED AND CHECKED BY FILE		

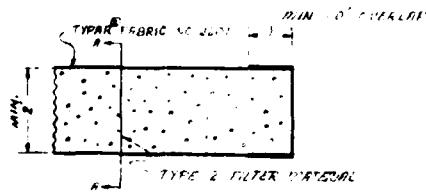
PLATE VI

2

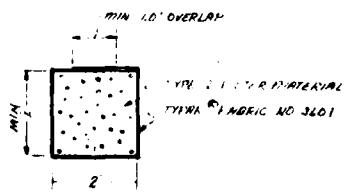


PROFILE @ "A" (LOOKING E)

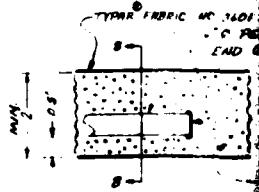
SCALE 1:50 4 10' V



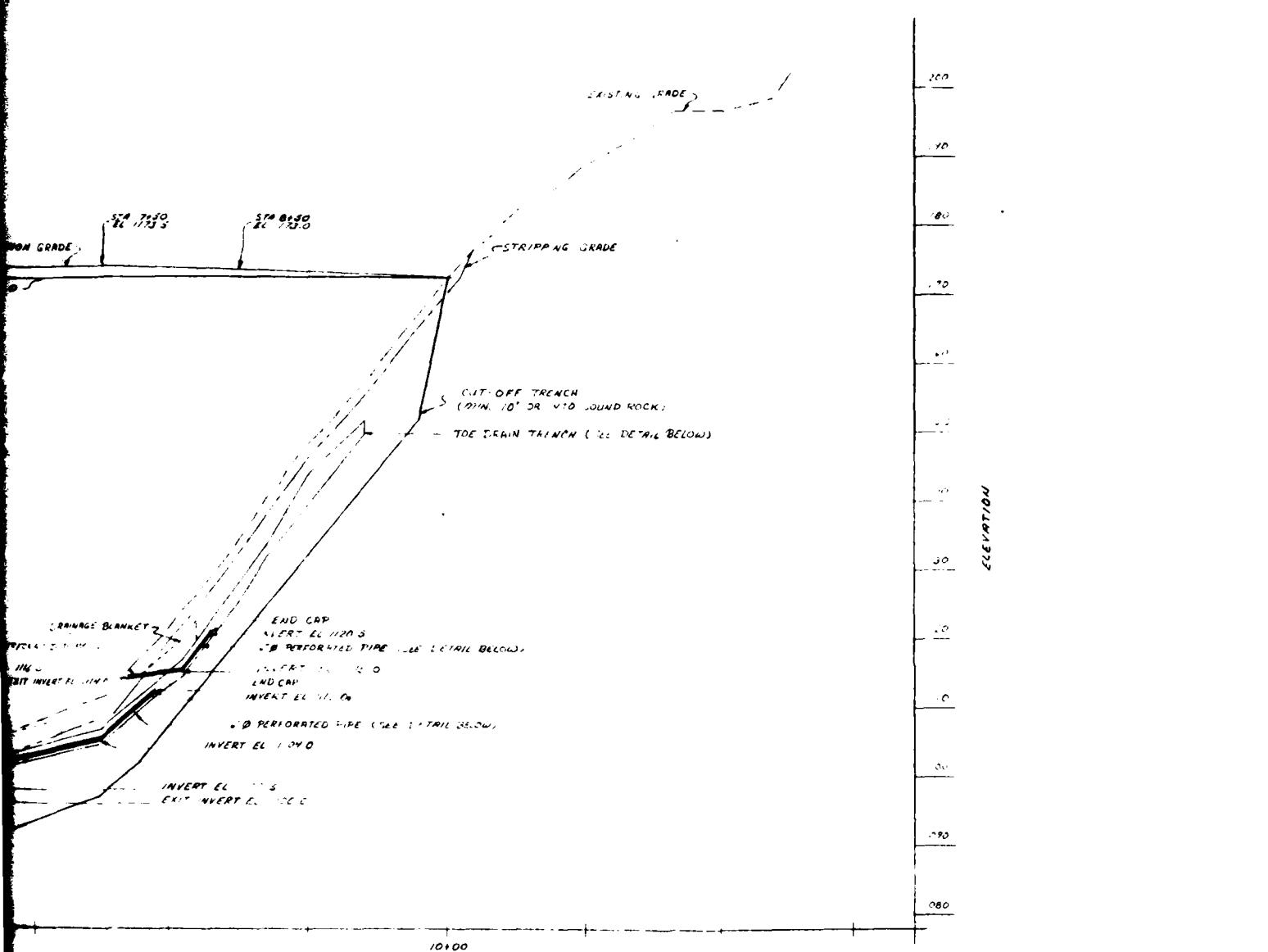
DETAIL TOE DRAIN TRENCH  
SCALE 1:2



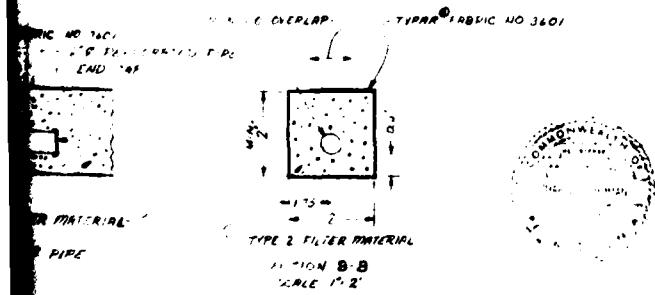
SECTION A-A  
SCALE 1" : 2'



DETAIL TOE DRAIN PIPE

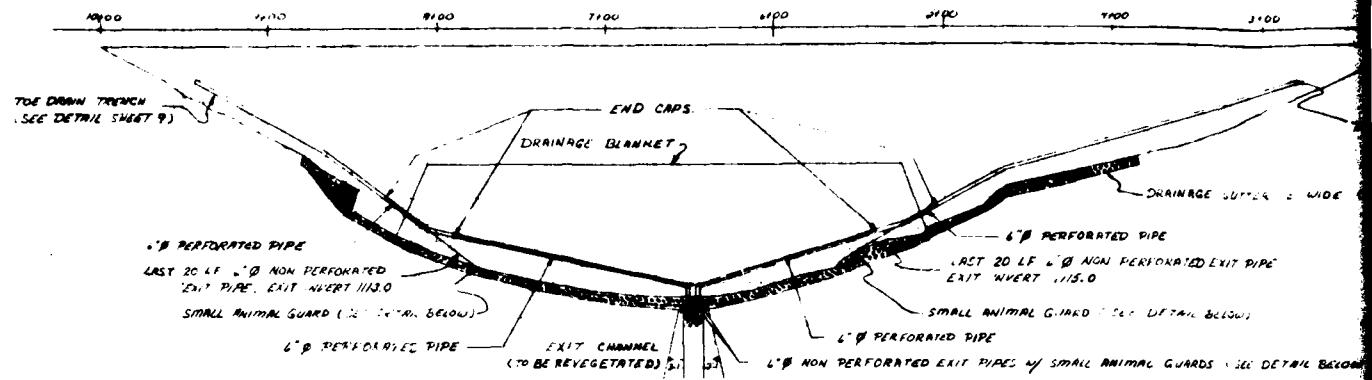


WORKING DOWNSTREAM)



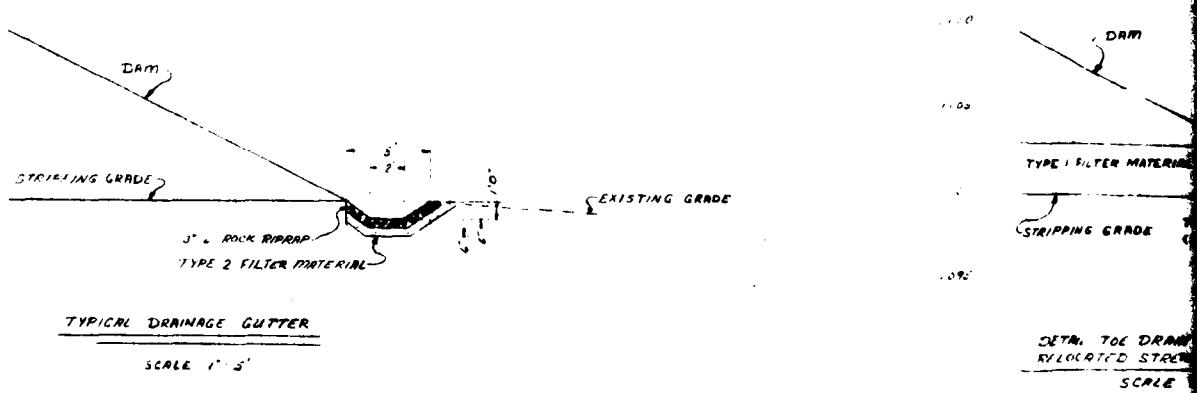
REVISIONS			BETHLEHEM MINES CORPORATION MINE NO. 60	
NO.	DATE	NAME	SLURRY IMPOUNDMENT CUT-OFF TRENCH AND TOE DRAIN PROFILE	
1	3/1/80	164/ARA	SHEET 9 OF 17	
			S.O. 13709-01-ARA	
			SCALE AS SHOWN	
			DATE MAY, 1980	
			MICHAEL BAKER, JR., INC. Consulting Engineers BEAVER, PENNSYLVANIA	
			DESIGNED J.G.B. DRAWN/TRADED 444 CHECKED <i>[initials]</i> FILE <i>[initials]</i>	

PLATE VII



DRAINAGE DETAIL PLAN VIEW

SCALE 1" : 50'



SCALE 1" : 5'

3100 2100 8' 8"

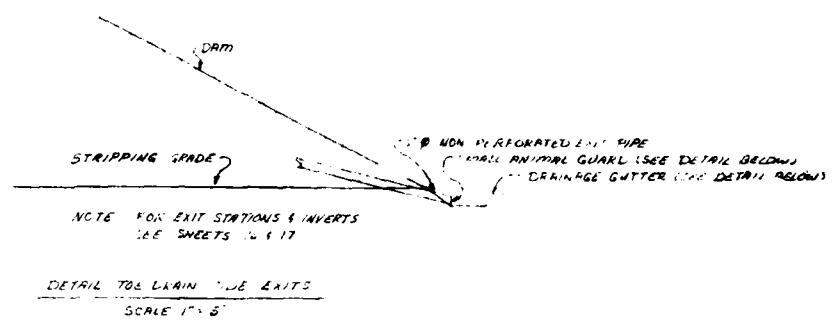
TOE DRAIN TRENCH (SEE DETAIL SHEET 9)

GUTTER 3' WIDE (SEE DETAIL BELOW)

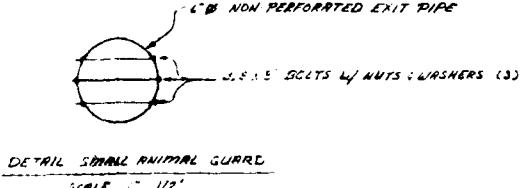
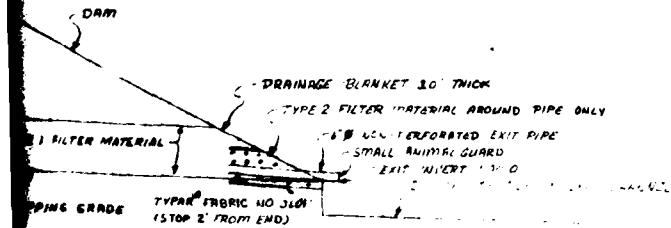
EXIT PIPE

BELCH

8' (SEE DETAIL BELOW)



DETAIL TOE DRAIN LINE EXIT  
SCALE 1" = 5'



DETAIL SMALL ANIMAL GUARD  
SCALE 1" = 1/2"

TOE DRAIN EXIT AT  
LOCATED STRAIGHT INVALY  
SCALE 1" = 5'



REVISIONS			BETHLEHEM MINES CORPORATION MINE NO. 60	
NO.	DATE	NAME	SLURRY IMPOUNDMENT DRAINAGE DETAILS	
			SHEET 10 OF 17	S.O. 13709-01-ARA
			SCALE AS SHOWN	DATE MAY, 1980
			MICHAEL BAKER, JR., INC. Consulting Engineers BEAVER, PENNSYLVANIA	DESIGNED J.G.U. DRAWN / TRACED AND CHECKED J.D. FILE

PLATE VIII

MICHAEL BAYER JR. INC		TEST BORING RECORD	
BORING NO. 100-1000-1000		DATE 10-10-1980	
PROJECT 100-1000-1000		LOCATION 100-1000-1000	
REMARKS		TEST BORING 100-1000-1000	
STATION	RIG	APP. DR. S. T. S.	AMBER S. T. S.
OFFSET	SAMPLER SIZE	HAMMER WT.	FALL S. T. S.
SURFACE	CASING SIZE	HAMMER WT.	FALL S. T. S.
DRILL RECORD		MATERIAL DESCRIPTION SOIL	
STATION	DRILLING DEPTH IN FEET	SOIL TYPE AND CLASSIFICATION	DEPTH IN FEET AND REMARKS
100-1000-1000	0-100	SOIL	100-1000-1000
	100-200	SOIL	
	200-300	SOIL	
	300-400	SOIL	
	400-500	SOIL	
	500-600	SOIL	
	600-700	SOIL	
	700-800	SOIL	
	800-900	SOIL	
	900-1000	SOIL	
	1000-1100	SOIL	
	1100-1200	SOIL	
	1200-1300	SOIL	
	1300-1400	SOIL	
	1400-1500	SOIL	
	1500-1600	SOIL	
	1600-1700	SOIL	
	1700-1800	SOIL	
	1800-1900	SOIL	
	1900-2000	SOIL	
	2000-2100	SOIL	
	2100-2200	SOIL	
	2200-2300	SOIL	
	2300-2400	SOIL	
	2400-2500	SOIL	
	2500-2600	SOIL	
	2600-2700	SOIL	
	2700-2800	SOIL	
	2800-2900	SOIL	
	2900-3000	SOIL	
	3000-3100	SOIL	
	3100-3200	SOIL	
	3200-3300	SOIL	
	3300-3400	SOIL	
	3400-3500	SOIL	
	3500-3600	SOIL	
	3600-3700	SOIL	
	3700-3800	SOIL	
	3800-3900	SOIL	
	3900-4000	SOIL	
	4000-4100	SOIL	
	4100-4200	SOIL	
	4200-4300	SOIL	
	4300-4400	SOIL	
	4400-4500	SOIL	
	4500-4600	SOIL	
	4600-4700	SOIL	
	4700-4800	SOIL	
	4800-4900	SOIL	
	4900-5000	SOIL	
	5000-5100	SOIL	
	5100-5200	SOIL	
	5200-5300	SOIL	
	5300-5400	SOIL	
	5400-5500	SOIL	
	5500-5600	SOIL	
	5600-5700	SOIL	
	5700-5800	SOIL	
	5800-5900	SOIL	
	5900-6000	SOIL	
	6000-6100	SOIL	
	6100-6200	SOIL	
	6200-6300	SOIL	
	6300-6400	SOIL	
	6400-6500	SOIL	
	6500-6600	SOIL	
	6600-6700	SOIL	
	6700-6800	SOIL	
	6800-6900	SOIL	
	6900-7000	SOIL	
	7000-7100	SOIL	
	7100-7200	SOIL	
	7200-7300	SOIL	
	7300-7400	SOIL	
	7400-7500	SOIL	
	7500-7600	SOIL	
	7600-7700	SOIL	
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## REVISIONS

DESIGNED

## DRAWING

CHECKED

REVIEWED 11-1-1956  
SO 13709-02-ARA

NOBTM

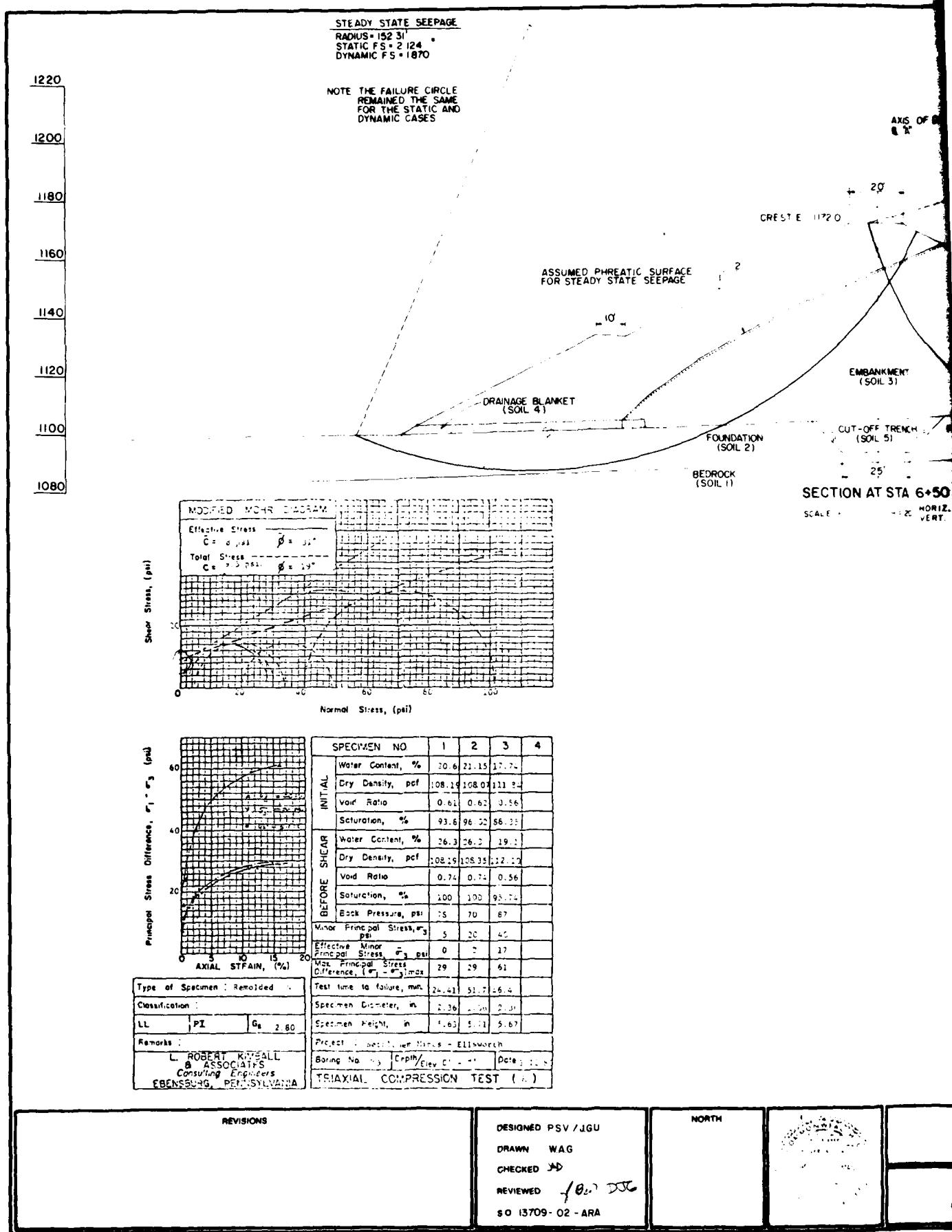
RF

MICHAEL BAKER, JR., INC.  
BORING NO. 1  
PROJECT: ELLIOTT MINE  
LOCATION: ELLIOTT MINE, 60 ft. N. of ELLIOTT  
BORING: Test Boring No. 1  
Boring Number: 1

TEST BORING RECORDS

STATION: 100 ft. from mine entrance  
DEPTH: 10 ft.  
SAMPLE: SAMPLE SIZE: 1 in. diameter  
MATERIAL: MATERIAL DESCRIPTION: SOIL  
DRAFT RECORDS: DRAFT RECORDS

GRAIN SIZE CLASSIFICATION	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644	645	646	647	648	649	650	651	652	653	654	655	656	657	658	659	660	661	662	663	664	665	666	667	668	669	670	671	672	673	674	675	676	677	678	679	680	681	682	683	684	685	686	687	688	689	690	691	692	693	694	695	696	697	698	699	700	701	702	703	704	705	706	707	708	709	710	711	712	713	714	715	716	717	718	719	720	721	722	723	724	725	726	727	728	729	730	731	732	733	734	735	736	737	738	739	740	741	742	743	744	745	746	747	748	749	750	751	752	753	754	755	756	757	758	759	760	761	762	763	764	765	766	767	768	769	770	771	772	773	774	775	776	777	778	779	780	781	782	783	784	785	786	787	788	789	790	791	792	793	794	795	796	797	798	799	800	801	802	803	804	805	806	807	808	809	810	811	812	813	814	815	816	817	818	819	820	821	822	823	824	825	826	827	828	829	830	831	832	833	834	835	836	837	838	839	840	841	842	843	844	845	846	847	848	849	850	851	852	853	854	855	856	857	858	859	860	861	862	863	864	865	866	867	868	869	870	871	872	873	874	875	876	877	878	879	880	881	882	883	884	885	886	887	888	889	890	891	892	893	894	895	896	897	898	899	900	901	902	903	904	905	906	907	908	909	910	911	912	913	914	915	916	917	918	919	920	921	922	923	924	925	926	927	928	929	930	931	932	933	934	935	936	937	938	939	940	941	942	943	944	945	946	947	948	949	950	951	952	953	954	955	956	957	958	959	960	961	962	963	964	965	966	967	968	969	970	971	972	973	974	975	976	977	978	979	980	981	982	983	984	985	986	987	988	989	990	991	992	993	994	995	996	997	998	999	1000	1001	1002	1003	1004	1005	1006	1007	1008	1009	1010	1011	1012	1013	1014	1015	1016	1017	1018	1019	1020	1021	1022	1023	1024	1025	1026	1027	1028	1029	1030	1031	1032	1033	1034	1035	1036	1037	1038	1039	1040	1041	1042	1043	1044	1045	1046	1047	1048	1049	1050	1051	1052	1053	1054	1055	1056	1057	1058	1059	1060	1061	1062	1063	1064	1065	1066	1067	1068	1069	1070	1071	1072	1073	1074	1075	1076	1077	1078	1079	1080	1081	1082	1083	1084	1085	1086	1087	1088	1089	1090	1091	1092	1093	1094	1095	1096	1097	1098	1099	1100	1101	1102	1103	1104	1105	1106	1107	1108	1109	1110	1111	1112	1113	1114	1115	1116	1117	1118	1119	1120	1121	1122	1123	1124	1125	1126	1127	1128	1129	1130	1131	1132	1133	1134	1135	1136	1137	1138	1139	1140	1141	1142	1143	1144	1145	1146	1147	1148	1149	1150	1151	1152	1153	1154	1155	1156	1157	1158	1159	1160	1161	1162	1163	1164	1165	1166	1167	1168	1169	1170	1171	1172	1173	1174	1175	1176	1177	1178	1179	1180	1181	1182	1183	1184	1185	1186	1187	1188	1189	1190	1191	1192	1193	1194	1195	1196	1197	1198	1199	1200	1201	1202	1203	1204	1205	1206	1207	1208	1209	1210	1211	1212	1213	1214	1215	1216	1217	1218	12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STEADY STATE SEEPAGE AND RAPID DRAWDOWN  
SOIL PARAMETERS

SOIL	TOTAL UNIT WEIGHT $\gamma_T$ (PCF)	EFFECTIVE STRESS - STRENGTH PARAMETERS	
		$\sigma$ (PSF)	$\phi$ (DEGREES)
1	145	3000	40
2	135	360	37.5
3	130	1152	31
4	135	0	38
5	130	1152	31

AXIS OF DAM

RAPID DRAWDOWN  
RADIUS = 106.3'  
E3 = 1.6'

EMERGENCY SPILLWAY CREST EL. 65.0

ASSUMED SATURATED SOIL  
FOR RAPID DRAWDOWN

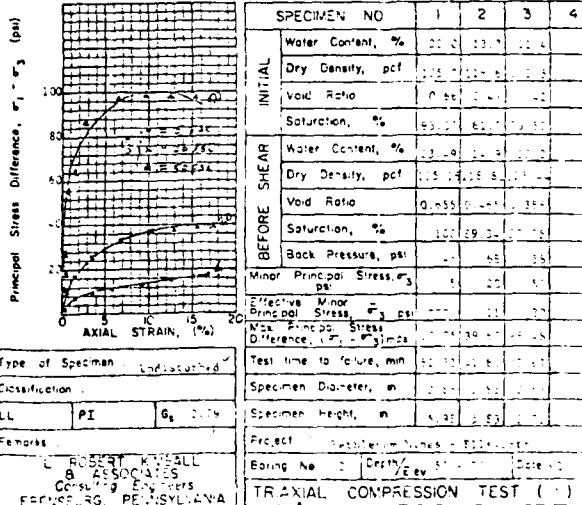
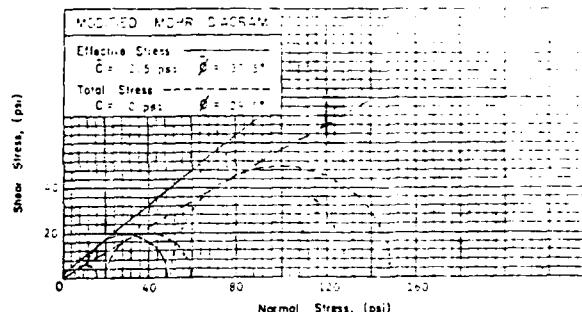
131

TRENCH 51 FOUNDATION (SOIL 2)

STA 6+50

25 428.2

24.4



BETHLEHEM MINES CORPORATION  
MINE NO 60  
SLURRY IMPOUNDMENT

MICHAEL BAKER, JR., INC.

CONSULTING ENGINEERS

BEAVER, PENNSYLVANIA



STABILITY ANALYSES AND  
TRIAXIAL SHEAR TEST RESULTS

SCALE 1 - 4-20 HORIZ VERT

DATE MAY, 1980

PLATE X

12

of 17

SHEET NO.

## CONSTRUCTION SPECIFICATIONS

### SITE PREPARATION

In preparing the site, all borrow areas and all areas under the embankment and any structural works shall be cleared, grubbed, and stripped of topsoil, trees, vegetation, rock or other objectionable material. In order to facilitate cleanout and re-vegetation, the entire work area will be cleared of all brush, trees, and debris and topsoil will be stockpiled at the designated location to be used for revegetation of the project area.

### CUT-OFF TRENCH

A cut-off trench shall be excavated along the centerline of the embankment. The depths of the cut-off trench on the drawings are approximate. Actual depths will be determined by the Owner's Representative after examination of materials encountered. The bottom of the excavation shall meet the requirements discussed under "Foundation Preparation." Side slopes shall be no steeper than 1V:4H. The trench shall be kept dewatered during the excavation and backfilling-compacting operations. The fill material for the cut-off trench shall consist primarily of clay (CL) material, obtained from the Borrow Area, and shall meet the requirements of the embankment material.

### EMBANKMENT

#### Material Classification

All excavation and embankment shall be un-classified. All material encountered regardless of hardness, type, durability and size shall be removed to the lines and grades as indicated on the Contract Drawings.

#### Suitability of Fill

Fill material shall be taken from approved borrow areas. Excavated material from the emergency spillway, cut-off trench excavation, and drainage trench excavation may be used for the embankment if approved by the Owner's Representative and placed in accordance with the specifications for the embankment. All fill material shall be clean soil, free of topsoil, roots, vegetation, stones or rock greater than 8 inches in diameter, frozen material or other objectionable material considered unsuitable by the Owner's Representative for use in embankment construction. In no case shall unburned coal processing waste be used. All borrow areas shall be adequately drained to provide a dry working area and the required moisture content of the embankment material.

#### Foundation Preparation

Immediately prior to placing the first layer of fill, all surfaces to receive fill shall be cleaned of all vegetation, debris, topsoil and other objectionable material. All roots shall be grubbed. All rock foundations shall be free of loose rock fragments and thoroughly cleaned to solid bed and sidewall to the satisfaction of the Owner's Representative. All rock foundations shall not have overhanging portions under which embankment material cannot be satisfactorily placed. After the necessary stripping and excavation has been completed, all earth foundations shall be moisture conditioned (sprinkled or scarified and dried) and compacted. The first lift of the embankment, except for the area of the drainage blanket, shall then be placed to a maximum thickness of 6 inches and thoroughly diced prior to compaction to provide a satisfactory bond between the first layer of fill and the earth foundation.

In the area of the former Lake Calydon where embankment is to be placed, the lake sediment deposits shall be totally removed. Final depth of stripping in this area shall be inspected by the Owner's Representative and approved prior to placement of the embankment. It is anticipated that this stripping will average approximately 3 feet deep.

#### Placement and Compaction

The combined borrow excavation and embankment placing operations shall be handled such that the materials when compacted in the embankment will be blended sufficiently to secure the best practicable degree of compaction, permeability, and stability. The distribution and gradation of materials throughout the embankment shall be handled such that the embankment will be free of coarse granular streaks or layers of material differing materially in texture or gradation from the surrounding material. The material shall be handled in the embankment in such a manner that segregation during placement is avoided. Coarse material with a greater rock content shall be placed in the lower areas of the embankment. All fill material shall be placed in

uniform horizontal layers of not more than 8 inches loose depth for the full width of the cross section. Fill material shall not be placed on surfaces that are muddy, frozen, or contain frost or ice. Each layer for its full width shall be compacted to not less than 95 percent of the maximum dry density at a moisture content not exceeding two percentage points above the optimum moisture content and not more than two percentage points below optimum. If necessary, each layer shall be moistened or saturated before compaction to provide the required moisture content of the soil material. The maximum dry density and optimum moisture content shall be determined in accordance with ASTM D-1557-78 (Modified Proctor Test).

### CREST PROTECTION

The crest of the embankment shall be protected with a coarse aggregate as shown in the construction drawings. The coarse aggregate shall be compacted to a depth of 5 inches and shall meet the requirements of PennDOT Form 408 Type 2A Aggregate.

### Field Quality Control

The subgrade and fill layers shall be inspected and approved by the Owner's Representative before further construction work may be performed thereon. To determine that the moisture content and dry density requirements of the compacted fill have been met, field and laboratory tests shall be made at frequent intervals by the Owner's Representative.

Prior to the earthwork operations, at least one laboratory compaction test will be performed in accordance with ASTM D-1557-78 for typical materials. During the earthwork operations, at least one additional laboratory compaction test will be performed for every material change or for every twenty (20) field tests. Compaction equipment is subject to the approval of the Owner's Representative. If fill compaction does not achieve specified densities due to existing compaction equipment, it shall be replaced with more effective equipment.

Field density tests will be performed for every 4,000 cubic yards of typical soil material in accordance with ASTM D1556-64 (1974), "Density of Soil In-Place by the Sand-Cone Method." A minimum of one field density test shall be performed for each day embankment fill is placed. Additional tests shall be performed where moisture is determined to vary substantially. Field determinations of the moisture content of soils will be in accordance with AASHTO T 217 67, "Determination of Moisture in Soils by Means of a Calcium Carbide Gas Pressure Moisture Tester."

Materials not meeting the specified moisture content and dry density requirements, as determined by these tests, shall be reworked until approved results are obtained. Material that cannot be dried to the specified moisture content before the next lift of fill is placed, shall be removed before continuing subsequent fill placement.

### DRAINAGE

#### Filter Fabric

The filter fabric shall be Type<sup>®</sup> Fabric No. 3601 or equivalent. Approval of alternate filter fabric is subject to prior notification of the Owner's Representative and approval by the Owner's Representative.

#### Type 1 Filter Material

Type 1 filter material shall meet the gradation requirements of PennDOT CC sand Type A as defined in Section 703.1 of the PennDOT Form 408 and shall be obtained from an approved source. It is the intent that this material will be placed adjacent to the existing site soils to act as a filter of the soil fines. The materials shall be reasonably uniform in physical properties and composed of hard, tough, durable, uncoated particles, thoroughly cleaned by washing, free from harmful amounts of clay, silt, vegetation, or other substances determined to be undesirable. The Type 1 filter material shall be natural sand resulting from glacial or water action. Limestone or slag fine aggregate is unacceptable.

#### Type 2 Filter Material

Type 2 filter material shall meet the gradation requirements of the PennDOT Form 408 Section 703.3 for Coarse Aggregate 1B and shall be obtained from an approved source. It is the intent that this material will be placed adjacent to the pipe perforations to prevent migration of material into the pipe. The materials shall be reasonably uniform in their physical properties and shall be composed of hard, tough, durable, uncoated particles, free of harmful amounts of

REVISIONS	DESIGNED	NORTH
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clay, silt, vegetation, or other substances determined to be undesirable. The Type 2 filter material shall be rounded, natural "pea" gravel resulting from glacial or water action. The crushed limestone aggregate, if used, shall be resistant to abrasion and water solutioning, and shall be free of fine material resulting from handling, in addition to the other requirements previously mentioned in this paragraph. Slag aggregate is unacceptable.

#### Drainage Trench

The drainage trench shall be installed to the lines and grades as shown on the drawings except where field conditions require that the trench be installed deeper when approved by the Owner's Representative. The trench shall be kept dewatered during the excavation, backfilling, and compacting operations.

#### Toe Drain Pipe

The 6-inch-diameter drain pipe shall be bituminous coated corrugated metal pipe, 16 gauge, perforated with 1/4" size perforations and shall meet the requirements of AASHTO Designation M 190-78, "Bituminous Coated Corrugated Metal Culvert Pipe and Pipe Arches." The pipe shall be fully coated. The size of the perforations shall be 1/4 inch. The pipe to carry the flow from the toe drain to the outlet channel shall be of the same gauge, size and requirement as specified above, but shall not be perforated. The upgrade ends of the drains shall be closed with commercially manufactured galvanized end caps. The discharge end of the pipe shall be suitably protected with an animal guard, either commercially available or as detailed on the drawings.

#### Drainage Blanket

The drainage blanket material, Type 1 Filter Material, shall meet the gradation requirements of PennDOT CC Sand Type A as defined in Section '03 1 of the PennDOT Form 408 and shall be obtained from an approved source. It is the intent that this material will provide adequate drainage of the downstream prism of the embankment and to act as a filter of the soil fines. The materials shall be reasonably uniform in physical properties and composed of hard, tough, durable, uncoated particles, thoroughly cleaned by washing, free from harmful amounts of clay, silt, vegetation, or other substances determined to be undesirable. The drainage blanket material shall be natural sand resulting from glacial or water action. Limestone or slag fine aggregate is unacceptable.

Prior to placing the first layer of the drainage blanket fill, the area shall be prepared in accordance with the requirements discussed under "Foundation Preparation," except that the first layer of drainage blanket fill shall not be disced and mixed with the foundation soil after initial compaction. The drainage blanket fill shall be placed in uniform horizontal layers of not more than 8 inches loose depth for the full width of the drainage blanket. Care shall be exercised by the Contractor to prevent contamination of the drainage blanket with other soils. If such contamination occurs it will be removed and replaced at the Contractor's expense. Drainage blanket fill shall not be placed on surfaces that are muddy, frozen, or contain frost or ice. Each layer for its full width shall be compacted to not less than 95 of the maximum dry density. The maximum dry density shall be determined in accordance with ASTM D-1557-78 (Modified Proctor Test).

#### Revegetation

The following seed mixture shall be used in revegetating the embankment, the entrance channel of the emergency spillway, and other areas disturbed during construction:

Mixture	Seeding Rate - lbs./acre
Perennial Ryegrass	20
Kentucky 31 Fescue	25
Crowfoot with inoculant	20

If the embankments are hydroseeded, the legume inoculant shall be added to the slurry just prior to seeding at 4 times the rate recommended on the package. Lime, fertilizer and hydromulch shall be applied with the seed application. Lime shall be applied at a rate of 2 tons per acre. Liming materials shall be agricultural ground limestone of sufficient fineness so that 98% will pass a 20-mesh sieve and 40% will pass a 100-mesh sieve. TSP (calcium carbonate equivalent) shall be 90 or higher. Six hundred pounds of 10-20-20 fertilizer shall be applied per acre at the time of seeding.

The embankments shall be mulched with hay mulch, blown onto seeded areas at a rate of 2.5 tons per acre. Mulch shall be anchored with asphalt spray applied at a rate of 80 gallons per acre.

#### ACCESS ROAD

The access road shall be blade cut in the field as shown in the construction drawings. The road surface shall be a coarse aggregate compacted to a depth of 5 inches and shall meet the requirements of PennDOT Form 408 Type 2A Aggregate.

#### RIPRAP

##### Surrounding of Material

Riprap shall consist of well graded limestone or sandstone aggregate obtained from an approved source. Riprap stone shall be dense, sound, resistant to abrasion, free of elongated and/or slabby stones, and free of cracks, seams, and other defects that would tend to increase unduly their destruction by water and frost actions. In no case shall coal-processing waste be used.

#### Placement

Riprap shall be placed to the lines and grades as shown on the construction plans and as directed for the protection of slopes, structures, gutters and channels. All riprap shall be placed in a manner to insure that the larger stones are uniformly distributed and the smaller stones serve to fill the spaces between the larger stones in such a manner as will result in well-keyed, densely placed mass of the specified thickness.

#### Bedding

In all areas, bedding, 6 inches in depth, shall be placed between the riprap and earth fill embankment or subgrade, to prevent water from removing fine material through voids in the stone. Bedding material shall be Type 1 filter material.

#### Gradation

Rock riprap sizes listed in the construction details are mean outside dimensions required and fifty percent of the stones shall be equal to or larger than the tabulated outside dimensions. All riprap shall meet the following gradation requirements:

Rock Riprap Size of Stone Required	Maximum Weight	Minimum and Maximum Weight of Stones	Weight Range of 5% of Stones
10 in	75 lbs	25-75 lbs	40-75 lbs
19 in	500 lbs	25-500 lbs	150-500 lbs

#### EROSION CONTROL

Construction operations shall be performed such that erosion and water pollution shall be minimized. The minimization shall take place by constructing a straw or hay bale barrier as shown on the Slurry Impoundment General Plan and construction drawings prior to construction. This barrier shall remain in place until revegetation of the embankment slopes and other disturbed areas has occurred. When sediment behind the barrier exceeds 10 foot in depth, the contractor shall remove the sediment and it shall be placed at a location upstream of the barrier in such a manner that it will not erode from the site.

#### MAINTENANCE

Revegetated areas will be inspected and limed and fertilized as required.

#### FIELD SURVEY INFORMATION

Elevation bench marks, coordinate control points, and project base lines shall be provided by Bethlehem Mines Corporation.

#### PROTECTION OF PERSONS AND PROPERTY

(a) The Contractor shall be responsible for initiating, maintaining and supervising all safety precautions and programs in connection with the work. The Contractor shall take all reasonable precautions for the safety of, and shall provide all reasonable protection to prevent damage, injury or loss to (i) employees, (ii) all other persons who may be affected, (iii) the work and all materials and equipment to be incorporated, (iv) other property at the site or in the vicinity thereof.

(b) The Contractor shall comply with all applicable laws, ordinances, rules, regulations, and orders of any public authority and Bethlehem Mines Corporation having jurisdiction for the safety of persons or property or to protect them from damage, injury or loss. He shall erect and maintain, as required by existing conditions and progress of the work, all reasonable safeguards for safety and protection including posting danger signs and other warnings against hazards, promulgating safety regulations and notifying owners and users of adjacent utilities.

## BETHLEHEM MINES CORPORATION

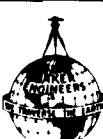
MINE NO. 60

SLURRY IMPOUNDMENT

MICHAEL BAKER, JR., INC.

CONSULTING ENGINEERS

BEAVER, PENNSYLVANIA



## CONSTRUCTION SPECIFICATIONS

SCALE: NONE

DATE: MAY, 1980

SHEET NO.

14

or 17

PLATE XI

APPENDIX F

GEOLOGY

## GEOLOGY

### Geomorphology

The Mine No. 60-Pond 6 Dam is located within the Pittsburgh Plateau section of the Appalachian Physiographic Province. This area is characterized by gently folded sedimentary rocks which have been incised by streams to form steep sided valleys. The site is located at the head of an unnamed tributary to the Center Branch of Pigeon Creek. The valley bottom of the unnamed tributary is about 200 feet below the adjacent hilltops. These rounded hilltops are at Elevation 1200 to 1300 feet, and in a regional sense are part of a broad, undulating plateau.

### Structure

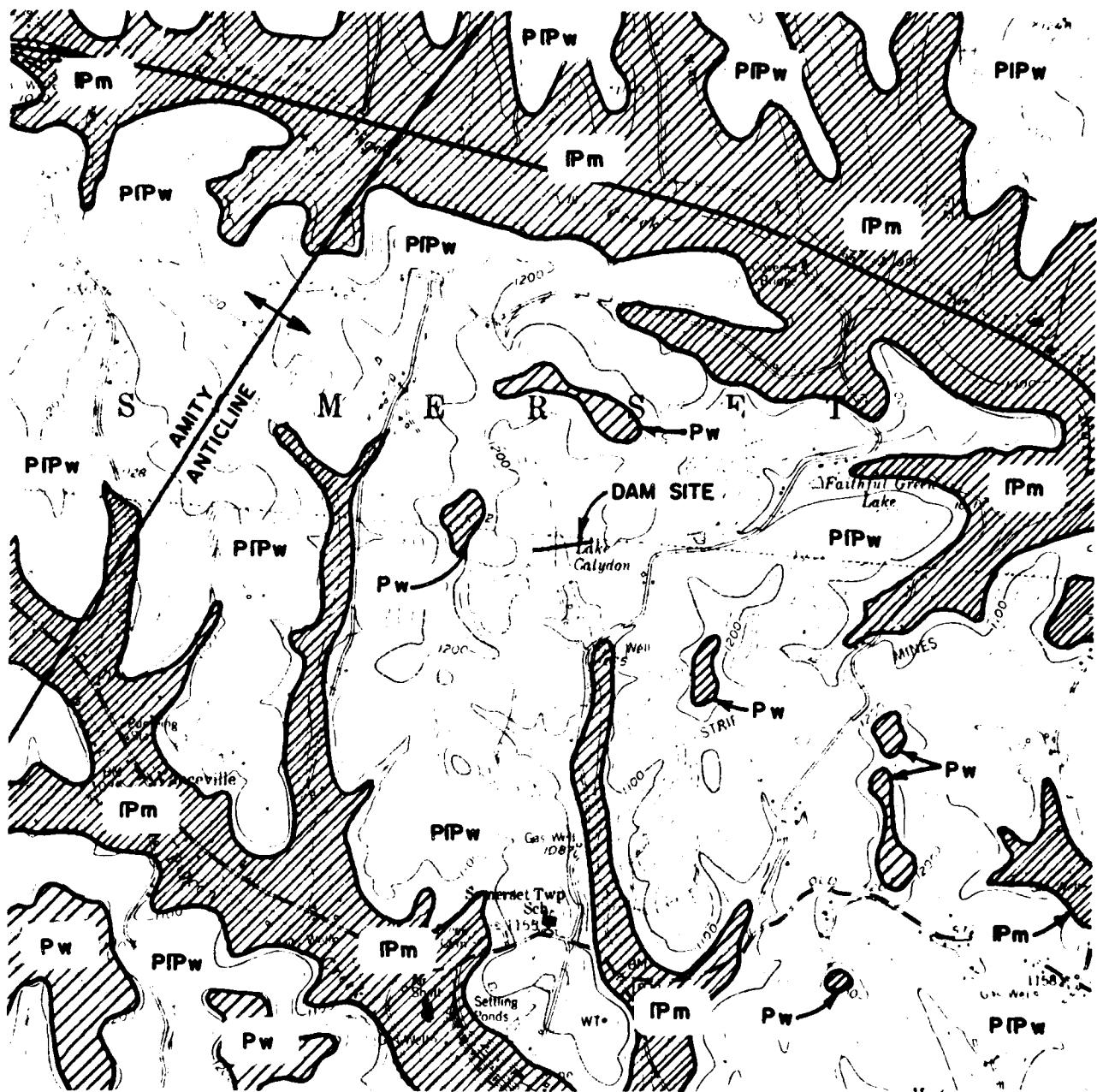
The site lies on the eastern flank of the Amity Anticline, the axis of which plunges to the southwest. Strata in the immediate vicinity of the dam dip to the south at an average rate of about  $0.3^\circ$ . Faulting has not been documented in the area of the dam and no observations were made that would indicate faulting in the rocks outcropping around the dam.

### Stratigraphy

Rocks outcropping in the immediate vicinity of the site belong to the Pennsylvanian Age, Casselman and Monongahela Formations and the Permian Age, Waynesburg and Washington Formations. The major rock types in all these formations are cyclic sequences of shale, limestone, sandstone, and coal.

### Mining Activity

The Pittsburgh Coal Seam, the lowermost unit of the Monongahela Formation, lies about 300 feet below the dam and has been extensively deep mined. The Waynesburg Coal Seam, which is the lowermost unit of the Waynesburg Formation, lies beneath the dam and has been unaffected by deep mining.



### HACKETT QUADRANGLE, WASHINGTON COUNTY, PENNSYLVANIA

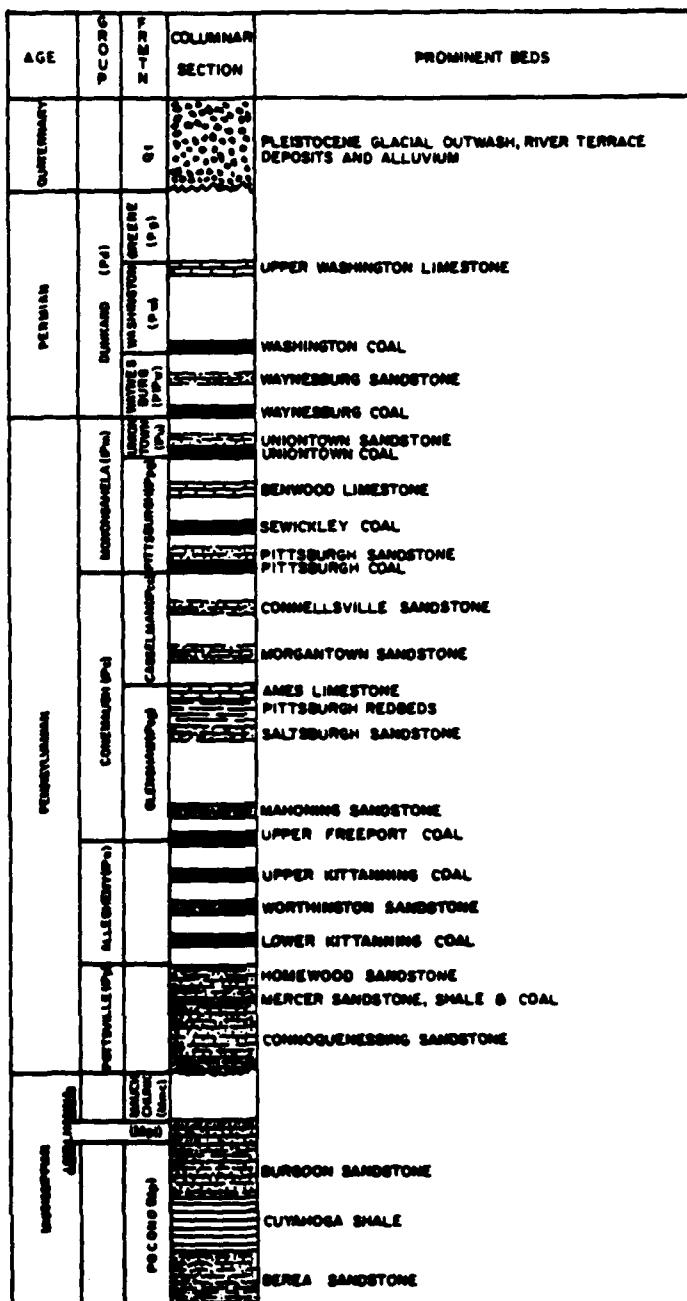
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CONTOUR INTERVAL 20 FT. DATUM IS MEAN SEA LEVEL  
FORMATION CONTACT



DATA OBTAINED FROM PENNSYLVANIA TOPOGRAPHIC AND GEOLOGIC SURVEY GREATER PITTSBURGH REGION GEOLOGIC MAP AND CROSS SECTIONS, 1975 and GREATER PITTSBURGH REGION STRUCTURE CONTOUR MAP, 1975

DATE: MAY 1981	SCALE: 1" = 2000'	MINE No. 60 - POND 6 NATIONAL DAM INSPECTION PROGRAM	GEOLOGIC MAP
DR: JF	CK:	ACKENHEIL & ASSOCIATES CONSULTING GEO SYSTEMS, INC. ENGINEERS 1000 BANKSVILLE RD / PITTSBURGH, PA 15216	



DATE: MAY 1981  
 SCALE: 1"=360  
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MINE No. 60 - POND 6  
 NATIONAL DAM INSPECTION PROGRAM  
 ACKENHEIL & ASSOCIATES CONSULTING  
 GEO SYSTEMS, INC. ENGINEERS  
 1000 BANKSVILLE RD/PITTSBURGH PA 15216

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